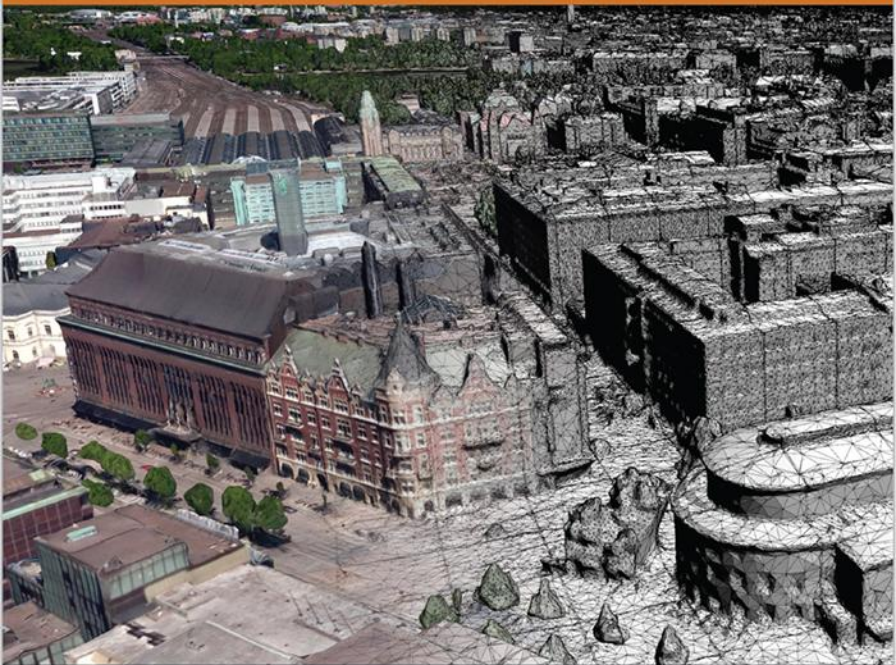


CHARLES D. GHILANI



ELEMENTARY SURVEYING

An Introduction to Geomatics

Sixteenth Edition



An aerial photograph of a city, likely Philadelphia, showing various buildings and streets. Overlaid on the image is a semi-transparent rectangular box containing the title. Additionally, a 3D wireframe model of a city block is visible in the foreground, partially overlapping the title box.

Elementary Surveying

An Introduction to Geomatics

Sixteenth Edition

CHARLES D. GHILANI

Professor Emeritus of Engineering
Pennsylvania State University



Pearson

Content Management: Dawn Murrin, Erin Ault
Content Development: Coleen Morrison
Content Production: Rajinder Singh, Pallavi Pandit, Kaitlin Smith
Product Management: Holly Stark
Product Marketing: Stacey Sveum, Wayne Stevens
Rights and Permissions: Anjali Singh

Please contact <https://support.pearson.com/getsupport/s/contactsupport> with any queries on this content

Cover Image: Courtesy of Bentley Systems, Incorporated.

Copyright © 2022, 2018, 2015 by Pearson Education, Inc. or its affiliates, 221 River Street, Hoboken, NJ 07030. All Rights Reserved. Manufactured in the United States of America. This publication is protected by copyright, and permission should be obtained from the publisher prior to any prohibited reproduction, storage in a retrieval system, or transmission in any form or by any means, electronic, mechanical, photocopying, recording, or otherwise. For information regarding permissions, request forms, and the appropriate contacts within the Pearson Education Global Rights and Permissions department, please visit www.pearsoned.com/permissions/

Acknowledgments of third-party content appear on the appropriate page within the text.

PEARSON, ALWAYS LEARNING, and Mastering[™] are exclusive trademarks owned by Pearson Education, Inc. or its affiliates in the U.S. and/or other countries.

Unless otherwise indicated herein, any third-party trademarks, logos, or icons that may appear in this work are the property of their respective owners, and any references to third-party trademarks, logos, icons, or other trade dress are for demonstrative or descriptive purposes only. Such references are not intended to imply any sponsorship, endorsement, authorization, or promotion of Pearson's products by the owners of such marks, or any relationship between the owner and Pearson Education, Inc., or its affiliates, authors, licensees, or distributors.

Library of Congress Cataloging-in-Publication Data

Names: Ghilani, Charles D., author.

Title: Elementary surveying : an introduction to geomatics / Charles Daniel Ghilani.

Description: Sixteenth edition. | Hoboken, N.J. : Pearson, 2022. | Includes bibliographical references and index.

Identifiers: LCCN 2020038712 | ISBN 9780136822820 (paperback)

ISBN 9780136822806 (rental edition)

Subjects: LCSH: Surveying. | Geomatics.

Classification: LCC TA545 .W77 2022 | DDC 526.9—dc23 LC record available at <https://lcn.loc.gov/2020038712>

ScoutAutomatedPrintCode

Rental

ISBN-10: 0-13-682280-0

ISBN-13: 978-0-13-682280-6

Print Offer

ISBN-10: 0-13-682282-7

ISBN-13: 978-0-13-682282-0





Table of Contents

PREFACE

XIX

Acknowledgments xxi

1 • INTRODUCTION

1

- 1.1 Definition of Surveying 1
- 1.2 Geomatics 3
- 1.3 History of Surveying 4
- 1.4 Geodetic and Plane Surveys 9
- 1.5 Importance of Surveying 10
- 1.6 Specialized Types of Surveys 11
- 1.7 Surveying Safety 12
- 1.8 Land and Geographic Information Systems 14
- 1.9 Federal Surveying and Mapping Agencies 15
- 1.10 The Surveying Profession 16
- 1.11 Professional Surveying Organizations 17
- 1.12 Surveying on the Internet 18
- 1.13 Future Challenges in Surveying 18
- Problems 20
- Bibliography 21

2 • UNITS, SIGNIFICANT FIGURES, AND FIELD NOTES

22

PART I UNITS AND SIGNIFICANT FIGURES 22

- 2.1 Introduction 22
- 2.2 Units of Measurement 22
- 2.3 International System of Units (SI) 24
- 2.4 Significant Figures 26
- 2.5 Rounding Off Numbers 28

PART II FIELD NOTES 29

- 2.6 Field Notes 29
- 2.7 General Requirements of Handwritten Field Notes 30
- 2.8 Types of Field Books 31
- 2.9 Kinds of Notes 32
- 2.10 Arrangements of Notes 32
- 2.11 Suggestions for Recording Notes 34
- 2.12 Introduction to Survey Controllers 35
- 2.13 Transfer of Files from Survey Controllers 38
- 2.14 Digital Data File Management 40
- 2.15 Advantages and Disadvantages of Survey Controllers 41
- Problems 42
- Bibliography 43

3 • THEORY OF ERRORS IN OBSERVATIONS

44

- 3.1 Introduction 44
- 3.2 Direct and Indirect Observations 44
- 3.3 Errors in Measurements 45
- 3.4 Mistakes 45
- 3.5 Sources of Errors in Making Observations 46
- 3.6 Types of Errors 46
- 3.7 Precision and Accuracy 47
- 3.8 Eliminating Mistakes and Systematic Errors 48
- 3.9 Probability 48
- 3.10 Most Probable Value 49
- 3.11 Residuals 50
- 3.12 Occurrence of Random Errors 50
- 3.13 General Laws of Probability 54
- 3.14 Measures of Precision 55
- 3.15 Interpretation of Standard Deviation 57
- 3.16 The 50%, 90%, and 95% Errors 57
- 3.17 Error Propagation 59
- 3.18 Applications 64
- 3.19 Conditional Adjustment of Observations 64
- 3.20 Weights of Observations 65

3.21	Least-Squares Adjustment	66
	Problems	67
	Bibliography	69

4 • LEVELING—THEORY, METHODS, AND EQUIPMENT 70

PART I LEVELING—THEORY AND METHODS 70

4.1	Introduction	70
4.2	Definitions	70
4.3	North American Vertical Datum	73
4.4	Curvature and Refraction	74
4.5	Methods for Determining Differences in Elevation	76

PART II EQUIPMENT FOR DIFFERENTIAL LEVELING 83

4.6	Categories of Levels	83
4.7	Telescopes	84
4.8	Level Vials	86
4.9	Tilting Levels	88
4.10	Automatic Levels	88
4.11	Digital Levels	90
4.12	Tripods	91
4.13	Hand Levels	91
4.14	Level Rods	92
4.15	Turning Points	94
4.16	Testing and Adjusting Levels	95
	Problems	99
	Bibliography	101

5 • LEVELING—FIELD PROCEDURES AND COMPUTATIONS 102

5.1	Introduction	102
5.2	Carrying and Setting Up a Level	102
5.3	Duties of a Rod Person	104
5.4	Differential Leveling	105
5.5	Precision	111
5.6	Adjustments of Simple Level Circuits	112
5.7	Reciprocal Leveling	113
5.8	Three-Wire Leveling	114
5.9	Profile Leveling	116
5.10	Grid, Cross-Section, or Borrow-Pit Leveling	120
5.11	Use of the Hand Level	121
5.12	Sources of Error in Leveling	121
5.13	Mistakes	123

5.14	Reducing Errors and Eliminating Mistakes	124
5.15	Using Software	124
	Problems	126
	Bibliography	128

6 • DISTANCE MEASUREMENT

129

PART I METHODS FOR MEASURING DISTANCES 129

6.1	Introduction	129
6.2	Summary of Methods for Making Linear Measurements	129
6.3	Pacing	130
6.4	Odometer Readings	130
6.5	Optical Rangefinders	130
6.6	Tacheometry	131
6.7	Subtense Bar	131

PART II DISTANCE MEASUREMENTS BY TAPING 131

6.8	Introduction to Taping	131
6.9	Taping Equipment and Accessories	132
6.10	Care of Taping Equipment	133
6.11	Taping on Level Ground	134
6.12	Horizontal Measurements on Sloping Ground	136
6.13	Slope Measurements	137
6.14	Sources of Error in Taping	139

PART III ELECTRONIC DISTANCE MEASUREMENT 143

6.15	Introduction	143
6.16	Propagation of Electromagnetic Energy	144
6.17	Principles of Electronic Distance Measurement	147
6.18	Electro-Optical Instruments	148
6.19	Total Station Instruments	151
6.20	EDM Instruments Without Reflectors	152
6.21	Computing Horizontal Lengths from Slope Distances	152
6.22	Errors in Electronic Distance Measurement	154
6.23	Using Software	161
	Problems	162
	Bibliography	163

7 • ANGLES, AZIMUTHS, AND BEARINGS

164

7.1	Introduction	164
7.2	Units of Angle Measurement	164
7.3	Kinds of Horizontal Angles	165

7.4	Direction of a Line	166
7.5	Azimuths	167
7.6	Bearings	168
7.7	Comparison of Azimuths and Bearings	169
7.8	Computing Azimuths	171
7.9	Computing Bearings	173
7.10	The Compass and the Earth's Magnetic Field	174
7.11	Magnetic Declination	176
7.12	Variations in Magnetic Declination	176
7.13	Software for Determining Magnetic Declination	178
7.14	Local Attraction	179
7.15	Typical Magnetic Declination Problems	180
7.16	Mistakes	182
	Problems	182

8 • TOTAL STATION INSTRUMENTS; ANGLE OBSERVATIONS **185**

PART I TOTAL STATION INSTRUMENTS 185

8.1	Introduction	185
8.2	Characteristics of Total Station Instruments	185
8.3	Functions Performed by Total Station Instruments	188
8.4	Parts of a Total Station Instrument	189
8.5	Handling and Setting Up a Total Station Instrument	192
8.6	Servo-Driven and Remotely Operated Total Station Instruments	195

PART II ANGLE OBSERVATIONS 196

8.7	Relationship of Angles and Distances	196
8.8	Observing Horizontal Angles with Total Station Instruments	197
8.9	Observing Multiple Horizontal Angles by the Direction Method	200
8.10	Closing the Horizon	201
8.11	Observing Deflection Angles	202
8.12	Observing Azimuths	204
8.13	Observing Vertical Angles	204
8.14	Sights and Marks	206
8.15	Prolonging a Straight Line	208
8.16	Balancing-in	209
8.17	Random Traverse	209
8.18	Total Stations for Determining Elevation Differences	211
8.19	Adjustment of Total Station Instruments and Their Accessories	212
8.20	Sources of Error in Total Station Work	216
8.21	Propagation of Random Errors in Angle Observations	222
8.22	Mistakes	223
	Problems	224
	Bibliography	225

9 • TRAVERSING**226**

- 9.1 Introduction 226
- 9.2 Observation of Traverse Angles or Directions 228
- 9.3 Observation of Traverse Lengths 229
- 9.4 Selection of Traverse Stations 230
- 9.5 Referencing Traverse Stations 231
- 9.6 Traverse Field Notes 232
- 9.7 Angle Misclosure 233
- 9.8 Traversing with Total Station Instruments 234
- 9.9 Radial Traversing 236
- 9.10 Sources of Error in Traversing 237
- 9.11 Mistakes in Traversing 237
- Problems 237

10 • TRAVERSE COMPUTATIONS**239**

- 10.1 Introduction 239
- 10.2 Balancing Angles 240
- 10.3 Computation of Preliminary Azimuths Or Bearings 242
- 10.4 Departures and Latitudes 243
- 10.5 Departure and Latitude Closure Conditions 245
- 10.6 Traverse Linear Misclosure and Relative Precision 245
- 10.7 Traverse Adjustment 246
- 10.8 Rectangular Coordinates 249
- 10.9 Alternative Methods for Making Traverse Computations 250
- 10.10 Inversing 254
- 10.11 Computing Final Adjusted Traverse Lengths and Directions 255
- 10.12 Coordinate Computations in Boundary Surveys 257
- 10.13 Use of Open Traverses 259
- 10.14 State Plane Coordinate Systems 262
- 10.15 Traverse Computations Using Computers 263
- 10.16 Locating Blunders in Traverse Observations 263
- 10.17 Mistakes in Traverse Computations 266
- Problems 266
- Bibliography 269

11 • COORDINATE GEOMETRY IN SURVEYING CALCULATIONS**270**

- 11.1 Introduction 270
- 11.2 Coordinate Forms of Equations for Lines and Circles 271
- 11.3 Perpendicular Distance from a Point to a Line 273
- 11.4 Intersection of Two Lines, Both Having Known Directions 275

11.5	Intersection of a Line With a Circle	277
11.6	Intersection of Two Circles	280
11.7	Three-Point Resection	282
11.8	Two-Dimensional Conformal Coordinate Transformation	285
11.9	Inaccessible Point Problem	290
11.10	Three-Dimensional Two-Point Resection	292
11.11	Software	295
	Problems	296
	Bibliography	300

12 • AREA

301

12.1	Introduction	301
12.2	Methods of Measuring Area	301
12.3	Area by Division Into Simple Figures	302
12.4	Area by Offsets from Straight Lines	303
12.5	Area by Coordinates	305
12.6	Area by Double-Meridian Distance Method	309
12.7	Area of Parcels with Circular Boundaries	312
12.8	Partitioning of Lands	313
12.9	Area by Measurements from Maps	317
12.10	Software	319
12.11	Sources of Error in Determining Areas	320
12.12	Mistakes in Determining Areas	320
	Problems	320

13 • GLOBAL NAVIGATION SATELLITE SYSTEMS—INTRODUCTION AND PRINCIPLES OF OPERATION

323

13.1	Introduction	323
13.2	Overview of GPS	324
13.3	The GPS Signal	326
13.4	Reference Coordinate Systems	329
13.5	Fundamentals of Satellite Positioning	339
13.6	Errors in Observations	341
13.7	Differential Positioning	349
13.8	Kinematic Methods	352
13.9	Relative Positioning	352
13.10	Precise Point Positioning	355
13.11	Other Satellite Navigation Systems	357
13.12	The Future	359
	Problems	361
	Bibliography	362

14 • GLOBAL NAVIGATION SATELLITE SYSTEMS—STATIC SURVEYS **363**

- 14.1 Introduction 363
- 14.2 Field Procedures in Static GNSS Surveys 364
- 14.3 Planning Satellite Surveys 369
- 14.4 Performing Static Surveys 380
- 14.5 Data Processing and Analysis 381
- 14.6 Things to Consider 391
- 14.7 A Method for Obtaining Orthometric Height Differences Using GNSS 394
- 14.8 Sources of Errors in Satellite Surveys 396
- 14.9 Mistakes in Satellite Surveys 398
- Problems 399
- Bibliography 401

15 • GLOBAL NAVIGATION SATELLITE SYSTEMS—KINEMATIC SURVEYS **402**

- 15.1 Introduction 402
- 15.2 Planning of Kinematic Surveys 403
- 15.3 Initialization Techniques 405
- 15.4 Equipment Used in Kinematic Surveys 406
- 15.5 Methods Used in Kinematic Surveys 408
- 15.6 Performing Post-Processed Kinematic Surveys 411
- 15.7 Communication in Real-Time Kinematic Surveys 414
- 15.8 Real-Time Networks 415
- 15.9 Performing Real-Time Kinematic Surveys 417
- 15.10 Machine Guidance and Control 419
- 15.11 Errors in Kinematic Surveys 421
- 15.12 Mistakes in Kinematic Surveys 422
- Problems 422
- Bibliography 423

16 • ADJUSTMENTS BY LEAST SQUARES **425**

- 16.1 Introduction 425
- 16.2 Fundamental Condition of Least Squares 427
- 16.3 Least-Squares Adjustment by the Observation Equation Method 428
- 16.4 Matrix Methods in Least-Squares Adjustment 432
- 16.5 Matrix Equations for Precisions of Adjusted Quantities 434
- 16.6 Least-Squares Adjustment of Leveling Circuits 436
- 16.7 Propagation of Errors 440
- 16.8 Least-Squares Adjustment of GNSS Baseline Vectors 442
- 16.9 Least-Squares Adjustment of Conventional Horizontal Plane Surveys 447
- 16.10 The Error Ellipse 456

16.11	Adjustment Procedures	461
16.12	Other Measures of Precision for Horizontal Stations	462
16.13	Software	465
16.14	Conclusions	465
	Problems	466
	Bibliography	471

17 • MAPPING SURVEYS

472

17.1	Introduction	472
17.2	Basic Methods for Performing Mapping Surveys	473
17.3	Map Scale	474
17.4	Control for Mapping Surveys	475
17.5	Contours	476
17.6	Characteristics of Contours	479
17.7	Method of Locating Contours	480
17.8	Digital Elevation Models and Automated Contouring Systems	482
17.9	Basic Field Methods for Locating Topographic Details	483
17.10	Planning a Laser-Scanning Survey	494
17.11	Three-Dimensional Conformal Coordinate Transformation	496
17.12	Selection of Field Method	498
17.13	Working With Survey Controllers and Field-to-Finish Software	498
17.14	Hydrographic Surveys	501
17.15	Sources of Error in Mapping Surveys	506
17.16	Mistakes in Mapping Surveys	506
	Problems	507
	Bibliography	508

18 • MAPPING

510

18.1	Introduction	510
18.2	Availability of Maps and Related Information	511
18.3	National Mapping Program	511
18.4	Accuracy Standards for Mapping	512
18.5	Manual and Computer-Aided Drafting Procedures	516
18.6	Map Design	517
18.7	Map Layout	519
18.8	Basic Map Plotting Procedures	520
18.9	Contour Interval	522
18.10	Plotting Contours	523
18.11	Lettering	524
18.12	Cartographic Map Elements	525
18.13	Drafting Materials	527
18.14	Automated Mapping and Computer-Aided Drafting Systems	528
18.15	Migrating Maps Between Software Packages	533
18.16	Impacts of Modern Land and Geographic Information Systems on Mapping	534

18.17	The Importance of Metadata	535
18.18	Sources of Error in Mapping	535
18.19	Mistakes in Mapping	535
	Problems	536
	Bibliography	538

19 • CONTROL SURVEYS AND GEODETIC REDUCTIONS

539

19.1	Introduction	539
19.2	The Ellipsoid and Geoid	540
19.3	The Conventional Terrestrial Pole	542
19.4	Geodetic Position and Ellipsoidal Radii of Curvature	544
19.5	Geoid Undulation and Deflection of the Vertical	547
19.6	U.S. Reference Frames	549
19.7	Transforming Coordinates Between Reference Frames	556
19.8	Accuracy Standards and Specifications for Control Surveys	561
19.9	The National Spatial Reference System	564
19.10	Hierarchy of the National Horizontal Control Network	564
19.11	Hierarchy of the National Vertical Control Network	565
19.12	Control Point Descriptions	565
19.13	Field Procedures for Conventional Horizontal Control Surveys	569
19.14	Field Procedures for Vertical-Control Surveys	574
19.15	Reduction of Field Observations to their Geodetic Values	579
19.16	Geodetic Position Computations	591
19.17	The Local Geodetic Coordinate System	594
19.18	Three-Dimensional Coordinate Computations	596
19.19	Software	598
	Problems	599
	Bibliography	601

20 • STATE PLANE COORDINATES AND OTHER MAP PROJECTIONS

603

20.1	Introduction	603
20.2	Projections Used in State Plane Coordinate Systems	604
20.3	Lambert Conformal Conic Projection	608
20.4	Transverse Mercator Projection	609
20.5	State Plane Coordinates in Nad 27 and Nad 83	610
20.6	Computing SPCS 83 Coordinates in the Lambert Conformal Conic System	611
20.7	Computing SPCS 83 Coordinates in the Transverse Mercator System	616
20.8	Reduction of Distances and Angles to State Plane Coordinate Grids	622
20.9	Computing State Plane Coordinates of Traverse Stations	632
20.10	Surveys Extending from One Zone to Another	635
20.11	The Universal Transverse Mercator Projection	636
20.12	Other Map Projections	637

20.13	Ground versus Grid Problem	641
20.14	Proposed Changes to SPCS in 2022	647
20.15	Map Projection Software	652
	Problems	654
	Bibliography	656

21 • BOUNDARY SURVEYS

657

21.1	Introduction	657
21.2	Categories of Land Surveys	658
21.3	Historical Perspectives	659
21.4	Property Description by Metes and Bounds	660
21.5	Property Description by Block-and-Lot System	663
21.6	Property Description by Coordinates	665
21.7	Retracement Surveys	665
21.8	Subdivision Surveys	668
21.9	Partitioning Land	670
21.10	Registration of Title	671
21.11	Adverse Possession and Easements	672
21.12	Condominium Surveys	672
21.13	Geographic and Land Information Systems	679
21.14	Sources of Error in Boundary Surveys	679
21.15	Mistakes	679
	Problems	680
	Bibliography	682

22 • SURVEYS OF THE PUBLIC LANDS

683

22.1	Introduction	683
22.2	Instructions for Surveys of the Public Lands	684
22.3	Initial Point	687
22.4	Principal Meridian	688
22.5	Baseline	688
22.6	Standard Parallels (Correction Lines)	690
22.7	Guide Meridians	690
22.8	Township Extérieurs, Meridional (Range) Lines, and Latitudinal (Township) Lines	690
22.9	Designation of Townships	692
22.10	Subdivision of a Quadrangle into Townships	692
22.11	Subdivision of a Township into Sections	693
22.12	Subdivision of Sections	695
22.13	Fractional Sections	695
22.14	Notes	696
22.15	Outline of Subdivision Steps	696
22.16	Marking Corners	698
22.17	Witness Corners	698
22.18	Meander Corners	698

22.19	Lost and Obliterated Corners	699
22.20	Accuracy of Public Land Surveys	701
22.21	Descriptions by Township Section, and Smaller Subdivision	702
22.22	BLM Land Information System	703
22.23	Sources of Error	704
22.24	Mistakes	704
	Problems	704
	Bibliography	706

23 • CONSTRUCTION SURVEYS

707

23.1	Introduction	707
23.2	Specialized Equipment for Construction Surveys	708
23.3	Horizontal and Vertical Control	712
23.4	Staking Out a Pipeline	713
23.5	Staking Pipeline Grades	714
23.6	Computing the Bend Angles in Pipelines	716
23.7	Staking Out a Building	717
23.8	Staking Out Highways	721
23.9	Other Construction Surveys	726
23.10	Construction Surveys Using Total Station Instruments	727
23.11	Construction Surveys Using GNSS Equipment	730
23.12	Machine Guidance and Control	732
23.13	As-Built Surveys with Laser Scanning	734
23.14	Sources of Error in Construction Surveys	735
23.15	Mistakes	735
	Problems	735
	Bibliography	737

24 • HORIZONTAL CURVES

738

24.1	Introduction	738
24.2	Degree of Circular Curve	739
24.3	Definitions and Derivation of Circular Curve Formulas	741
24.4	Circular Curve Stationing	743
24.5	General Procedure of Circular Curve Layout by Deflection Angles	744
24.6	Computing Deflection Angles and Chords	746
24.7	Notes for Circular Curve Layout by Deflection Angles and Incremental Chords	748
24.8	Detailed Procedures for Circular Curve Layout by Deflection Angles and Incremental Chords	749
24.9	Setups on Curve	750
24.10	Metric Circular Curves by Deflection Angles and Incremental Chords	751
24.11	Circular Curve Layout by Deflection Angles and Total Chords	753
24.12	Computation of Coordinates on a Circular Curve	753
24.13	Circular Curve Layout by Coordinates	755

24.14	Curve Stakeout Using GNSS Receivers and Robotic Total Stations	760
24.15	Circular Curve Layout by Offsets	761
24.16	Special Circular Curve Problems	764
24.17	Compound and Reverse Curves	765
24.18	Sight Distance on Horizontal Curves	766
24.19	Spirals	767
24.20	Computation of "As-Built" Circular Alignments	771
24.21	Sources of Error in Laying Out Circular Curves	774
24.22	Mistakes	775
	Problems	775
	Bibliography	777

25 • VERTICAL CURVES

778

25.1	Introduction	778
25.2	General Equation of a Vertical Parabolic Curve	779
25.3	Equation of an Equal Tangent Vertical Parabolic Curve	780
25.4	High or Low Point on a Vertical Curve	782
25.5	Vertical Curve Computations Using the Tangent-Offset Equation	782
25.6	Equal Tangent Property of a Parabola	786
25.7	Curve Computations by Proportion	787
25.8	Staking a Vertical Parabolic Curve	787
25.9	Machine Control in Grading Operations	788
25.10	Computations for an Unequal Tangent Vertical Curve	788
25.11	Designing a Curve to Pass Through a Fixed Point	790
25.12	Sight Distance	792
25.13	Sources of Error in Laying Out Vertical Curves	794
25.14	Mistakes	794
	Problems	794
	Bibliography	796

26 • VOLUMES

797

26.1	Introduction	797
26.2	Methods of Volume Measurement	797
26.3	The Cross-Section Method	798
26.4	Types of Cross Sections	799
26.5	Average-End-Area Formula	800
26.6	Determining End Areas	801
26.7	Computing Slope Intercepts	804
26.8	Prismoidal Formula	806
26.9	Volume Computations	808
26.10	Unit-Area, or Borrow-Pit, Method	809
26.11	Contour-Area Method	811
26.12	Measuring Volumes of Water Discharge	812
26.13	Software	813
26.14	Sources of Error in Determining Volumes	814

26.15	Mistakes	814
	Problems	815
	Bibliography	817

27 • PHOTOGRAMMETRY

818

27.1	Introduction	818
27.2	Uses of Photogrammetry	819
27.3	Aerial Cameras	820
27.4	Types of Aerial Photographs	822
27.5	Vertical Aerial Photographs	824
27.6	Scale of a Vertical Photograph	825
27.7	Ground Coordinates from a Single Vertical Photograph	828
27.8	Relief Displacement on a Vertical Photograph	830
27.9	Flying Height of a Vertical Photograph	832
27.10	Stereoscopic Parallax	833
27.11	Stereoscopic Viewing	836
27.12	Stereoscopic Measurement of Parallax	837
27.13	Analytical Photogrammetry	839
27.14	Stereoscopic Plotting Instruments	839
27.15	Orthophotos	844
27.16	Ground Control for Photogrammetry	845
27.17	Flight Planning	846
27.18	Airborne Laser-Mapping Systems	848
27.19	Small Unmanned Aerial Systems	849
27.20	Remote Sensing	851
27.21	Software	856
27.22	Sources of Error in Photogrammetry	858
27.23	Mistakes	858
	Problems	858
	Bibliography	861

28 • INTRODUCTION TO GEOGRAPHIC INFORMATION SYSTEMS

862

28.1	Introduction	862
28.2	Land Information Systems	864
28.3	GIS Data Sources and Classifications	865
28.4	Spatial Data	865
28.5	Nonspatial Data	871
28.6	Data Format Conversions	871
28.7	Creating GIS Databases	874
28.8	Metadata	880
28.9	GIS Analytical Functions	880
28.10	GIS Applications	884
28.11	Data Sources	885
	Problems	887
	Bibliography	889

APPENDIX A • TAPE CORRECTION PROBLEMS 890

- A.1 Correcting Systematic Errors in Taping 890

APPENDIX B • EXAMPLE NOTEFORMS 893

APPENDIX C • ASTRONOMIC OBSERVATIONS 900

- C.1 Introduction 900
- C.2 Overview of Usual Procedures for Astronomical Azimuth Determination 901
- C.3 Ephemerides 903
- C.4 Definitions 906
- C.5 Time 908
- C.6 Timing Observations 910
- C.7 Computations for Azimuth from Polaris Observations by the Hour Angle Method 911
- C.8 Azimuth from Solar Observations 913
- C.9 Importance of Precise Leveling 915

APPENDIX D • USING THE WORKSHEETS FROM THE COMPANION WEBSITE 916

- D.1 Introduction 916
- D.2 Using the Files 916
- D.3 Worksheets as an Aid in Learning 921

APPENDIX E • INTRODUCTION TO MATRICES 922

- E.1 Introduction 922
- E.2 Definition of a Matrix 922
- E.3 The Dimensions of a Matrix 923
- E.4 The Transpose of a Matrix 924
- E.5 Matrix Addition 924
- E.6 Matrix Multiplication 924
- E.7 Matrix Inverse 926

**APPENDIX F • U.S. STATE PLANE COORDINATE
SYSTEM DEFINING PARAMETERS 928**

- F.1 Introduction 928
- F.2 Defining Parameters for States Using the Lambert Conformal Conic Map
Projection 928
- F.3 Defining Parameters for States Using the Transverse Mercator Map
Projection 931

APPENDIX G • ANSWERS TO SELECTED PROBLEMS 933

**APPENDIX H • COMMONLY USED CONVERSIONS
AND ABBREVIATIONS 938**

INDEX 944



Preface

This 16th edition of *Elementary Surveying: An Introduction to Geomatics* presents basic concepts and practical material in each of the areas fundamental to modern surveying (geomatics) practice. It is written primarily for students beginning their study of surveying (geomatics) at the college level. Although the book is introductory to the practice of surveying, its depth and breadth also make it ideal for self-study and preparation for licensing examinations. This edition includes more than 400 figures and illustrations to help clarify discussions, and numerous example problems are worked to illustrate computational procedures.

In keeping with the goal of providing an up-to-date presentation of surveying equipment and procedures, total stations are stressed as the instruments for making angle and distance observations. In the chapters on GNSS surveys, discussions have been included to bring awareness to the inaccuracies of directions obtained from a GNSS survey along with the effect of centering errors on horizontal positioning. Chapter 17 introduces the reader to unmanned systems and Chapter 18 has a section on the importance of metadata. Chapter 20 on mapping the ground versus grid problem is discussed including subsections on low-distortion projections and single project factor use.

As with past editions, this book continues to emphasize the theory of errors in surveying work. At the end of each chapter, common errors and mistakes related to the topic covered are listed so that students will be reminded to exercise caution in all of their work. Practical suggestions resulting from the author's many years of experience are interjected throughout the text. Many of the 1,000 end-of-chapter problems have been rewritten so that instructors can create new assignments for their students. An Instructor's Manual is available on the instructor resource center for instructors who adopt the book by contacting their Pearson sales representative.

Updated versions of STATS, WOLFPACK, and MATRIX are available in Mastering Engineering. These programs contain options for statistical computations, traverse computations for polygon, link, and radial traverses; area calculations; astronomical azimuth reduction; two-dimensional coordinate transformations; horizontal and vertical curve computations; and least-squares adjustments. Mathcad[®] worksheets and Excel[®] spreadsheets are included on the companion website. These programmed computational sheets demonstrate the solution to many of the example problems discussed herein. For those desiring additional knowledge in map projections, the Mercator, Albers Equal Area, Oblique Stereographic, and Oblique Mercator map projections have been included with these files.

RESOURCES FOR INSTRUCTORS

All instructor resources are available for download at www.pearson.com. If you are in need of a login and password for this site, please contact your local Pearson representative.

Instructor's Solutions Manual

Available to adopters of this textbook, it contains the solutions to the homework problems. The Solutions Manual is available in PDF format.

PowerPoint Slides

A complete set of all the figures and tables from the textbook are available in PowerPoint format.

Mastering Engineering

This online tutorial and assessment program allows you to integrate dynamic homework with automated grading of the calculation parts of problems and personalized feedback. Mastering[™] Engineering allows you to easily track the performance of your entire class on an assignment-by-assignment basis, or the detailed work of an individual student. For more information visit www.masteringengineering.com.

WHAT'S NEW IN THIS EDITION

- Discussion on use and limitations of small unmanned aerial systems (sUAS).
- Discussion on the importance of metadata.
- Discussion on GNSS Precise Point Positioning.
- Discussion on the new gravimetric- and geometric-based datums for the United States in 2022.
- Discussion on the computational changes to the state plane coordinate systems in 2022.
- Discussion on the planned vertical datum for the North American continent involving Canada, Mexico, and the United States.
- Discussion on the planned new horizontal and vertical datums.

- Discussion on the ground versus grid problem with map projections including low-distortion projections.
- Revised problems sets.

■ ACKNOWLEDGMENTS

Past editions of this book, and this current one, have benefited from the suggestions, reviews, and other input from numerous students, educators, and practitioners. For their help the author is extremely grateful. In this edition, those professors and graduate students who reviewed material or otherwise assisted include Robert Schultz, Oregon State University; Steven Frank and James Reilly, New Mexico State University; Jeremy Deal, University of Texas-Arlington; Eric Fuller, St. Cloud State University; Loren J. Gibson, Florida Atlantic University; John J. Rose, Phoenix College; Robert Moynihan, University of New Hampshire; Marlee Walton, Iowa State University; Douglas E. Smith, Montana State University; Jean M. Rüeger, The University of New South Wales, Sydney, Australia; Thomas Seybert, Frank Derby, and Brian Naberezny, Pennsylvania State University; Paul Dukas and Bon DeWitt, University of Florida; Preston Hartzell, University of Houston; Ryan White, University of New Brunswick; Heather Nicholson; Thomas Meyer, University of Connecticut; Craig Rollins, National Geospatial Intelligence Agency; Dru Smith, Daniel Gillins, and Michael Dennis, National Geodetic Survey. The author would like to acknowledge the following professionals for their contributions and suggestions including Charles Harpster, Pennsylvania Department of Transportation; Eduardo Fernandez-Falcon, Topcon Positioning Systems; Joseph Gabor, Thomas Meyer, the University of Connecticut, and Dru Smith, Michael Dennis, and Dan Gillins from the National Geodetic Survey.

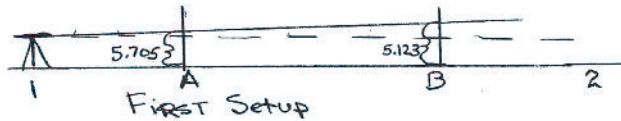
In addition, the author wishes to acknowledge the contributions of charts, maps, or other information from the National Geodetic Survey, the U.S. Geological Survey, and the U.S. Bureau of Land Management. Also appreciation is expressed to the many instrument manufacturers who provided images and other descriptive information on their equipment for used herein. To all of those named above, and to any others who may have been inadvertently omitted, the author is extremely thankful.

Combine this...

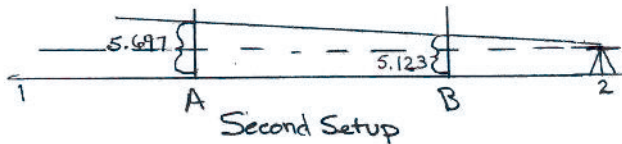
Hwk: 4.32

Intro to Surveying

Annie Ghilani



$$\Delta \text{Elev}_{AB} = (5.123 + 2E) - (5.705 + E)$$



$$\Delta \text{Elev}_{AB} = (5.123 + E) - (5.697 + 2E)$$

So $(5.123 + 2E) - (5.705 + E) = (5.123 + E) - (5.697 + 2E)$

Solving for E

$$-0.582 + E = -0.574 - E$$

$$2E = -0.574 + 0.582$$

$$2E = 0.008$$

$$E = 0.004$$

Check

$$\text{Setup 1: } [5.123 + 2(0.004)] - [5.705 + 0.004] = -0.578 \quad \checkmark$$

$$\text{Setup 2: } (5.123 + 0.004) - [5.697 + 2(0.004)] = -0.578 \quad \checkmark$$

With the Power of Mastering Engineering for Elementary Surveying

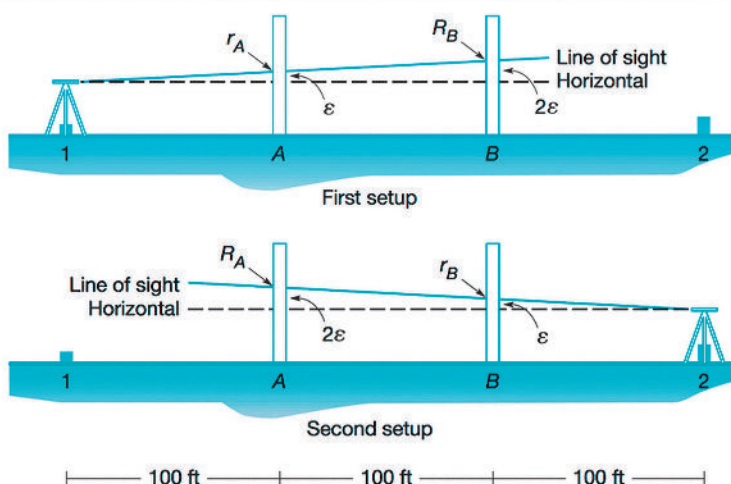
Mastering™ is the teaching and learning platform that empowers every student. By combining trusted authors' content with digital tools developed to engage students and emulate the office hours experience, Mastering personalizes learning and improves results for each student.

Problem 4.32 - Enhanced - with Hints and Feedback

< 1 of 1 >

Review

A horizontal collimation test is performed on an automatic level, using four locations lying in a straight line on approximately level ground, as shown.



With the instrument setup at point 1, the rod reading at A was 5.705 ft. and to B it was 5.123 ft. After moving and leveling the instrument at point 2, the rod reading to A was 5.697 ft and to B was 5.123 ft.

Part A

What is the collimation error of the instrument?

Express your answer to three significant figures and include appropriate units.

View Available Hint(s)

Hint 1. Finding the collimation error

The collimation error, ϵ , which here is the vertical error over 100 ft of horizontal distance, is calculated from the results of the collimation test using the formula $\epsilon = \frac{R_B - r_A - r_B + R_A}{2}$.

Hint 2. Using the formula

Submit Previous Answers

Correct

If the collimation error were to be reported in terms of ϵ per unit sight distance, you would divide by 100 ft to obtain $\epsilon = -4 \times 10^{-5} \text{ ft/ft}$.

This page intentionally left blank

An aerial photograph of a city, likely New York City, showing a dense urban landscape. A semi-transparent wireframe mesh is overlaid on a portion of the city, highlighting a specific building and its surrounding area. The wireframe is composed of numerous small, interconnected triangles, creating a 3D effect. The background shows various city buildings, streets, and green spaces.

1

Introduction

■ 1.1 DEFINITION OF SURVEYING

Surveying, which is also interchangeably called *geomatics* (see Section 1.2), has traditionally been defined as the science, art, and technology of determining the relative positions of points above, on, or beneath the Earth's surface, or of establishing such points. In a more general sense, however, surveying (geomatics) can be regarded as that discipline that encompasses all methods for measuring and collecting information about the physical earth and our environment, processing that information, and disseminating a variety of resulting products to a wide range of clients. Surveying has been important since the beginning of civilization. Its earliest applications were in measuring and marking boundaries of property ownership. Throughout the years, its importance has steadily increased with the growing demand for a variety of maps and other spatially related types of information, and the expanding need for establishing accurate line and grade to guide construction operations.

Today the importance of measuring and monitoring our environment is becoming increasingly critical as our population expands, land values appreciate, our natural resources dwindle, and human activities continue to stress the quality of our land, water, and air. Using modern ground, aerial, and satellite technologies, and computers for data processing, contemporary surveyors are now able to measure and monitor the Earth and its natural resources on literally a global basis. Never before has so much information been available for assessing current conditions, making sound planning decisions, and formulating policy in a host of land-use, resource development, and environmental preservation applications.

Recognizing the increasing breadth and importance of the practice of surveying, the *International Federation of Surveyors* (see Section 1.11) adopted the following definition:

"A surveyor is a professional person with the academic qualifications and technical expertise to conduct one, or more, of the following activities;

- to determine, measure and represent the land, three-dimensional objects, point-fields, and trajectories;
- to assemble and interpret land and geographically related information;
- to use that information for the planning and efficient administration of the land, the sea and any structures thereon; and
- to conduct research into the above practices and to develop them.

Detailed Functions

The surveyor's professional tasks may involve one or more of the following activities, which may occur either on, above, or below the surface of the land or the sea and may be carried out in association with other professionals.

1. The determination of the size and shape of the earth and the measurements of all data needed to define the size, position, shape and contour of any part of the earth and monitoring any change therein.
2. The positioning of objects in space and time as well as the positioning and monitoring of physical features, structures and engineering works on, above or below the surface of the earth.
3. The development, testing, and calibration of sensors, instruments, and systems for the above-mentioned purposes and for other surveying purposes.
4. The acquisition and use of spatial information from close range, aerial and satellite imagery and the automation of these processes.
5. The determination of the position of the boundaries of public or private land, including national and international boundaries, and the registration of those lands with the appropriate authorities.
6. The design, establishment, and administration of geographic information systems (GIS), and the collection, storage, analysis, management, display and dissemination of data.
7. The analysis, interpretation, and integration of spatial objects and phenomena in GIS, including the visualization and communication of such data in maps, models, and mobile digital devices.
8. The study of the natural and social environment, the measurement of land and marine resources and the use of such data in the planning of development in urban, rural, and regional areas.
9. The planning, development and redevelopment of property, whether urban or rural and whether land or buildings.
10. The assessment of value and the management of property, whether urban or rural and whether land or buildings.
11. The planning, measurement and management of construction works, including the estimation of costs.

In application of the foregoing activities surveyors take into account the relevant legal, economic, environmental, and social aspects affecting each project."

The breadth and diversity of the practice of surveying (geomatics), as well as its importance in modern civilization, are readily apparent from this definition.

■ 1.2 GEOMATICS

As noted in Section 1.1, geomatics is a term that is now commonly being applied to encompass the areas of practice formerly identified as surveying. The principal reason cited for making the name change is that the manner and scope of practice in surveying have changed dramatically in recent years. This has occurred in part because of recent technological developments that have provided surveyors with new tools for measuring and/or collecting information, for computing, and for displaying and disseminating information. It has also been driven by increasing concerns about the environment locally, regionally, and globally, which have greatly exacerbated efforts in monitoring, managing, and regulating the use of our land, water, air, and other natural resources. These circumstances, and others, have brought about a vast increase in demands for new spatially related information.

Historically surveyors made their measurements using ground-based methods using the transit and tape¹ as their primary instruments. Computations, analyses, and the reports, plats, and maps they delivered to their clients were prepared (in hardcopy form) through tedious manual processes. Today's surveyor has an arsenal of tools for measuring and collecting environmental information that includes electronic instruments for automatically measuring distances and angles, satellite surveying systems for quickly obtaining precise positions of widely spaced points, modern aerial digital imaging, and laser-scanning systems for quickly mapping and collecting other forms of data about the Earth, and, as shown in Figure 1.1, small unmanned systems to carry sensors such as digital cameras, laser scanners, and sonar. In addition, computer systems are available that can process the measured data and automatically produce plats, maps, and other products at speeds unheard of a few years ago. Furthermore, these products can be prepared in electronic formats and be transmitted to and from remote locations via telecommunication systems.

Concurrent with the development of these new data collection and processing technologies, *geographic information systems* (GISs) have matured. These



Figure 1.1
Unmanned
aerial system.
(Courtesy of Leica
Geosystems, Inc.)

¹These instruments are described in Appendix A and Chapter 6, respectively.

computer-based systems enable virtually any type of spatially related information about the environment to be integrated, analyzed, displayed, and disseminated.² The key to successfully operating GIS is spatially related data of high quality, and the collection and processing of this data placing great new demands upon the surveying community.

As a result of these new developments noted above, and others, many feel that the name surveying no longer adequately reflects the expanded and changing role of their profession. Hence the new term geomatics has emerged. In this text, the terms surveying and geomatics are both used, although the former is used more frequently. Nevertheless, students should understand that the two terms are synonymous as discussed above. Additionally due to the many diverse fields in what was traditionally called surveying, the entire field is now part of what is known as the *geospatial industries*.

■ 1.3 HISTORY OF SURVEYING

The oldest historical records in existence today that bear directly on the subject of surveying state that this science began in Egypt. Herodotus recorded that Sesostris (about 1400 B.C.) divided the land of Egypt into plots for the purpose of taxation. Annual floods of the Nile River swept away portions of these plots, and surveyors were appointed to replace the boundaries. These early surveyors were called *rope-stretchers*, since their measurements were made with ropes having markers at unit distances.

As a consequence of this work, early Greek thinkers developed the science of geometry. Their advance, however, was chiefly along the lines of pure science. Heron stands out prominently for applying science to surveying in about 120 B.C. He was the author of several important treatises of interest to surveyors, including *The Dioptra*, which related the methods of surveying a field, drawing a plan, and making related calculations. It also described one of the first pieces of surveying equipment recorded, the *dioptra* [Figure 1.2(a)]. For many years, Heron's work was the most authoritative among Greek and Egyptian surveyors.

Significant development in the art of surveying came from the practical-minded Romans, whose best-known writing on surveying was by Frontinus. Although the original manuscript disappeared, copied portions of his work have been preserved. This noted Roman engineer and surveyor, who lived in the 1st century, was a pioneer in the field, and his essay remained the standard for many years. The engineering ability of the Romans was demonstrated by their extensive construction work throughout the empire. Surveying necessary for this construction resulted in the organization of a surveyors' guild. Ingenious instruments were developed and used. Among these were the *groma* [Figure 1.2(b)], used for sighting; the *libella*, an A-frame with a plumb bob, for leveling; and the *chorobates*, a horizontal straightedge about 20-ft long with supporting legs and a groove on top for water to serve as a level.

²Geographic information systems are briefly introduced in Section 1.9, and then described in greater detail in Chapter 28.

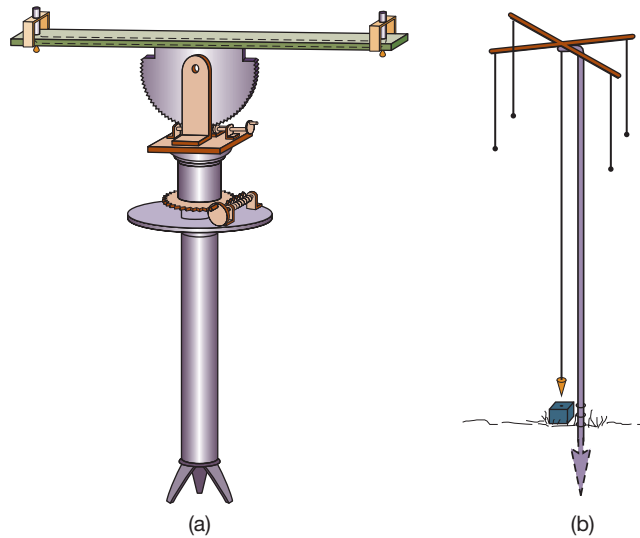


Figure 1.2
Historical surveying
instruments: (a) the
dioptra and (b) the
groma.

One of the oldest Latin manuscripts in existence is the *Codex Acerianus*, written in about the 6th century. It contains an account of surveying as practiced by the Romans and includes several pages from Frontinus's treatise. The manuscript was found in the 10th century by Gerbert and served as the basis for his text on geometry, which was largely devoted to surveying.

During the Middle Ages, the Arabs kept Greek and Roman science alive. Little progress was made in the art of surveying, and the only writings pertaining to it were called "practical geometry."

In the 13th century, Von Piso wrote *Practica Geometria*, which contained instructions on surveying. He also authored *Liber Quadratorum*, dealing chiefly with the *quadrans*, a square brass frame having a 90° angle and other graduated scales. A movable pointer was used for sighting. Other instruments of the period were the *astrolabe*, a metal circle with a pointer hinged at its center and held by a ring at the top, and the *cross staff*, a wooden rod about 4-ft long with an adjustable crossarm at right angles to it. The known lengths of the arms of the cross staff permitted distances to be measured by proportion and angles.

Early civilizations assumed the Earth to be a flat surface, but by noting the Earth's circular shadow on the moon during lunar eclipses and watching ships gradually disappear as they sailed toward the horizon, it was slowly deduced that the planet actually curved in all directions.

Determining the true size and shape of the Earth has intrigued humans for centuries. History records that a Greek named Eratosthenes was among the first to compute its dimensions. His procedure, which occurred about 200 B.C., is illustrated in Figure 1.3. Eratosthenes had concluded that the Egyptian cities of Alexandria and Syene were located approximately on the same meridian, and he had also observed that at noon on the summer solstice, the sun was directly overhead at Syene. (This was apparent because at that time of that day, the image of the sun could be seen reflecting from the bottom of a deep vertical well

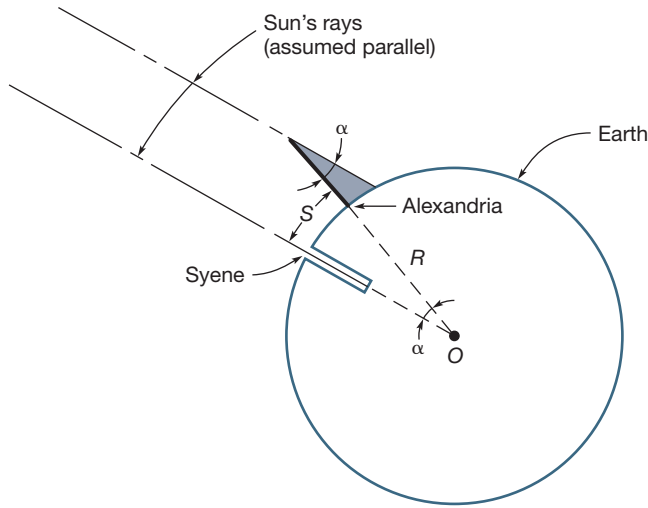


Figure 1.3
Geometry of the
procedure used
by Eratosthenes
to determine
the Earth's
circumference.

there.) He reasoned that at that moment, the sun, Syene, and Alexandria were in a common meridian plane, and if he could measure the arc length between the two cities, and the angle it subtended at the Earth's center, he could compute the Earth's circumference. He determined the angle by measuring the length of the shadow cast at Alexandria from a vertical staff of known length. The arc length was found from multiplying the number of caravan days between Syene and Alexandria by the average daily distance traveled. From these measurements, Eratosthenes calculated the Earth's circumference to be about 25,000 mi. Subsequent precise geodetic measurements using better instruments, but techniques similar geometrically to Eratosthenes', have shown his value, though slightly too large, to be amazingly close to the currently accepted one. (Actually, as explained in Chapter 19, the Earth approximates an oblate spheroid having an equatorial radius about 13.5 mi longer than the polar radius.)

In the 18th and 19th centuries, the art of surveying advanced more rapidly. The need for maps and locations of national boundaries caused England and France to make extensive surveys requiring accurate triangulation; thus, geodetic surveying began. The U.S. Coast Survey (now the National Geodetic Survey of the U.S. Department of Commerce) was established by an act of Congress in 1807. Initially its charge was to perform hydrographic surveys and prepare nautical charts. Later its activities were expanded to include establishment of reference monuments of precisely known positions throughout the country.

Increased land values and the importance of precise boundaries, along with the demand for public improvements in the canal, railroad, and turnpike eras, brought surveying into a prominent position. More recently, the large volume of general construction, numerous land subdivisions that require precise records, and demands posed by the fields of exploration and ecology have entailed an augmented surveying program. Surveying is still the sign of progress in the development, use, and preservation of the Earth's resources.

In addition to meeting a host of growing civilian needs, surveying has always played an important role in our nation's defense activities. World Wars I and II, the Korean and Vietnam conflicts, and the more recent conflicts in the Middle East, Europe, and Afghanistan have created staggering demands for precise measurements and accurate maps. These military operations also provided the stimulus for improving instruments and methods to meet these needs. Surveying also contributed to, and benefited from, the space program where new equipment and systems were needed to provide precise control for missile alignment, and for mapping and charting portions of the moon and nearby planets.

Developments in surveying and mapping equipment have now evolved to the point where the traditional instruments that were used until about the 1960s or 1970s—the transit, theodolite, dumpy level, and steel tape—have now been almost completely replaced by an array of new digital instruments. These include electronic *total station instruments*, which can be used to automatically measure and record horizontal and vertical distances, and horizontal and vertical angles; and *Global Navigation Satellite Systems* (GNSS) such as the *Global Positioning System* (GPS) that can provide precise location information for virtually any type of survey. Laser-scanning instruments combine automatic distance and angle measurements to compute dense grids of coordinated points. Also new aerial digital cameras and remote sensing instruments have been developed, which provide images in digital form, and these images can be processed to obtain spatial information and maps using new *digital photogrammetric restitution instruments* (also called *softcopy plotters*). Figures 1.4, 1.5, 1.6, and 1.7, respectively, show a total station instrument, 3D mobile mapping system, laser-scanning instrument, and modern softcopy plotter. The 3D mobile mapping system in Figure 1.5 is an integrated system consisting of scanners, GNSS-receiver, inertial measurement unit, and a high-quality hemispherical digital camera that can map all items within 100 m of the vehicle as the vehicle travels at highway speeds. The system can capture 1.3 million



Figure 1.4
LEICA TPS 1100
total station
instrument.
(Courtesy Leica
Geosystems, Inc.)

Figure 1.5

A mobile mapping system. (Courtesy Topcon Positioning Systems.)



Figure 1.6

A ground-based laser scanner. (Courtesy of Christopher Gibbons, Leica Geosystems, Inc.)



data points per second providing the end user with high-quality, georeferenced coordinates on all items visible in the images.

One area where laser scanning is finding great acceptance is in *Building Information Management* (BIM). In BIM, scanners are being used to capture the three-dimensional features of both the exterior and interior of existing buildings. The three-dimensional point cloud is then used by engineers, architects, and building contractors in developing plans for reconstruction projects or checking buildings for compliance with codes. Archeologists are also finding uses for laser scanning. By scanning ancient artifacts, not only is the artifact being preserved for future preservation projects, scans have also identified possible clues to how items were constructed and have even identified possible locations of buildings whose locations were previously unknown.



Figure 1.7
Intergraph Image
Station Z softcopy
plotter. (From
*Elements of
Photogrammetry:
With Applications
in GIS*, by Wolf
and Dewitt,
2000, Courtesy
Intergraph, Inc.,
and the McGraw-
Hill Companies.)

■ 1.4 GEODETIC AND PLANE SURVEYS

Two general classifications of surveys are *geodetic* and *plane*. They differ principally in the assumptions on which the computations are based, although field measurements for geodetic surveys are usually performed to a higher order of accuracy than those for plane surveys.

In geodetic surveying, the curved surface of the Earth is considered by performing the computations on an *ellipsoid* (curved surface approximating the size and shape of the Earth—see Chapter 19). It is now becoming common to do geodetic computations in a three-dimensional, *Earth-centered, Earth-fixed* (ECEF) Cartesian coordinate system. The calculations involve solving equations derived from solid geometry and calculus. Geodetic methods are employed to determine relative positions of widely spaced monuments and to compute lengths and directions of the long lines between them. These monuments serve as the basis for referencing other subordinate surveys of lesser extents. The modern reference systems are four-dimensional including the three-dimensional position of a point on the surface of the Earth with respect to time. These X , Y , H , and t systems take into account a location's movement due to ground motions caused by subsidence and uplift, plate tectonics, and other disturbing sources.

In early geodetic surveys, painstaking efforts were employed to accurately observe angles and distances. The angles were measured using precise ground-based theodolites, and the distances were measured using special tapes made from metal having a low coefficient of thermal expansion. From these basic measurements, the relative positions of the monuments were computed. Later, electronic instruments were used for observing the angles and distances. Although these latter types of instruments are still sometimes used on geodetic surveys, satellite positioning has now almost completely replaced other instruments for these types of surveys. Satellite positioning can provide the needed positions with much greater accuracy,

speed, and economy. GNSS receivers enable ground stations to be located precisely by observing distances to satellites operating in known positions along their orbits. GNSS surveys are being used in all forms of surveying including geodetic, hydrographic, construction, and boundary surveying. When combined with a real-time network (RTN), GNSS surveys are capable of providing accuracy within 0.1 ft over a 50-km region with as little as three minutes of data. The principles of operation of the global positioning system are given in Chapter 13, field and office procedures used in static GNSS surveys are discussed in Chapter 14, and the methods used in kinematic GNSS surveys including RTNs are discussed in Chapter 15.

In plane surveying, except for leveling, the reference base for fieldwork and computations is assumed to be a flat horizontal surface. The direction of a plumb line (and thus gravity) is considered parallel throughout the survey region, and all observed angles are presumed to be plane angles. For areas of limited size, the surface of our vast ellipsoid is actually nearly flat. On a line 5-mi line, the ellipsoid arc and chord lengths differ by only about 0.02 ft. A plane surface tangent to the ellipsoid departs only about 0.7 ft at 1 mi from the point of tangency. In a triangle having an area of 75 square miles, the difference between the sum of the three ellipsoidal angles and three plane angles is only about 1 second. Therefore, it is evident that except in surveys covering extensive areas, the Earth's surface can be approximated as a plane, thus simplifying computations and techniques. In general, algebra, plane and analytical geometry, and plane trigonometry are used in plane-surveying calculations. Even for very large areas, map projections, such as those described in Chapter 20, allow plane-surveying computations to be used. This book concentrates primarily on methods of plane surveying, an approach that satisfies the requirements of most projects.

■ 1.5 IMPORTANCE OF SURVEYING

Surveying is one of the world's oldest and most important arts because, as noted previously, from the earliest times it has been necessary to mark boundaries and divide land. Surveying has now become indispensable to our modern way of life. The results of today's surveys are used to (1) map the Earth above and below sea level; (2) prepare navigational charts for use in the air, on land, and at sea; (3) establish property boundaries of private and public lands; (4) develop data banks of land-use and natural resource information that aid in managing our environment; (5) determine facts on the size, shape, gravity, and magnetic fields of the earth; and (6) prepare charts of our moon and planets.

Surveying continues to play an extremely important role in many branches of engineering. For example, surveys are required to plan, construct, and maintain highways, railroads, rapid-transit systems, buildings, bridges, missile ranges, launching sites, tracking stations, tunnels, canals, irrigation ditches, dams, drainage works, urban land subdivisions, water supply and sewage systems, pipelines, and mine shafts. Surveying methods are commonly employed in laying out industrial assembly lines and jigs.³ These methods are also used for guiding the fabrication

³These instruments are described in Appendix A and Chapter 6, respectively.

of large equipment, such as airplanes and ships, where separate pieces that have been assembled at different locations must ultimately be connected as a unit. Surveying is important in many related tasks in agronomy, archeology, astronomy, forestry, geography, geology, geophysics, landscape architecture, meteorology, paleontology, and seismology, but particularly in military and civil engineering.

All engineers must know the limits of accuracy possible in construction, plant design and layout, and manufacturing processes, even though someone else may do the actual surveying. In particular, surveyors and civil engineers who are called on to design and plan surveys must have a thorough understanding of the methods and instruments used, including their capabilities and limitations. This knowledge is best obtained by making observations with the kinds of equipment used in practice to get a true concept of the theory of errors and the small but recognizable differences that occur in observed quantities.

In addition to stressing the need for reasonable limits of accuracy, surveying emphasizes the value of significant figures. Surveyors and engineers must know when to work to hundredths of a foot instead of to tenths or thousandths, or perhaps the nearest foot, and what precision in field data is necessary to justify carrying out computations to the desired number of decimal places. With experience, they learn how available equipment and personnel abilities govern procedures and results.

Engineers who design buildings, bridges, equipment, and so on are fortunate if their estimates of loads to be carried are correct within 5%. Then a factor of safety of 2 or more is often applied. However, except for some topographic work, only exceedingly small errors can be tolerated in surveying, and there is no factor of safety. Traditionally, therefore, both manual and computational precision are stressed in surveying.

■ 1.6 SPECIALIZED TYPES OF SURVEYS

Many types of surveys are so specialized that a person proficient in a particular discipline may have little contact with the other areas. Persons seeking careers in surveying and mapping, however, should be knowledgeable in every phase, since all are closely related in modern practice. Some important classifications are described briefly here.

Control surveys establish a network of horizontal and vertical monuments that serve as a reference framework for initiating other surveys. Many control surveys performed today are done using techniques discussed in Chapters 14 and 15 with GNSS instruments.

Topographic surveys determine locations of natural and artificial features and elevations used in map making.

Land, boundary, and cadastral surveys establish property lines and property corner markers. The term cadastral is now generally applied to surveys of the public lands systems. There are three major categories: *original surveys* to establish new section corners in unsurveyed areas that still exist in Alaska and several western states; *retracement surveys* to recover previously established boundary lines; and *subdivision surveys* to establish monuments and delineate new parcels of ownership. *Condominium surveys*, which provide a legal record of ownership, are a type of boundary survey.

Hydrographic surveys define shorelines and depths of lakes, streams, oceans, reservoirs, and other bodies of water. *Sea surveying* is associated with port and offshore industries and the marine environment, including measurements and marine investigations made by shipborne personnel.

Alignment surveys are made to plan, design, and construct highways, railroads, pipelines, and other linear projects. They normally begin at one control point and progress to another in the most direct manner permitted by field conditions.

Construction surveys provide line, grade, control elevations, horizontal positions, dimensions, and configurations for construction operations. They also secure essential data for computing construction pay quantities.

As-built surveys document the precise final locations and layouts of engineering works and record any design changes that may have been incorporated into the construction. These are particularly important when underground facilities are constructed, so their locations can be accurately known for maintenance purposes, and so that unexpected damage to them can be avoided during later installation of other underground utilities.

Mine surveys are performed above and below ground to guide tunneling and other operations associated with mining. This classification also includes geophysical surveys for mineral and energy resource exploration.

Solar surveys map property boundaries, solar easements, obstructions according to sun angles, and meet other requirements of zoning boards and title insurance companies.

Optical tooling (also referred to as *industrial surveying* or *optical alignment*) is a method of making extremely accurate measurements for manufacturing processes where small tolerances are required.

Except for control surveys, most other types described are usually performed using plane-surveying procedures, but geodetic methods may be employed on the others if a survey covers an extensive area or requires extreme accuracy.

Ground, aerial, and satellite surveys are broad classifications sometimes used. Ground surveys utilize measurements made with ground-based equipment such as automatic levels and total station instruments. Aerial surveys are accomplished using either *photogrammetry* or *remote sensing*. Photogrammetry uses cameras that are carried usually in airplanes to obtain images, whereas remote sensing employs cameras and other types of sensors that can be transported in either aircraft or satellites. Procedures for analyzing and reducing the image data are described in Chapter 27. Aerial methods have been used in all the specialized types of surveys listed, except for optical tooling, and in this area *terrestrial* (ground-based) photographs are often used. Satellite surveys include the determination of ground locations from measurements made to satellites using GNSS receivers, or the use of satellite images for mapping and monitoring large regions of the Earth.

■ 1.7 SURVEYING SAFETY

Surveyors (geomatics engineers) generally are involved in both field and office work. The fieldwork consists in making observations with various types of instruments to either (a) determine the relative locations of points or (b) to set out stakes in accordance with planned locations to guide building and construction

operations. The office work involves (1) conducting research and analysis in preparing for surveys, (2) computing and processing the data obtained from field measurements, and (3) preparing maps, plats, charts, reports, and other documents according to client specifications. Sometimes the fieldwork must be performed in hostile or dangerous environments, and thus it is very important to be aware of the need to practice safety precautions.

Among the most dangerous of circumstances within which surveyors must sometimes work are job sites that are either on or near highways or railroads, or that cross such facilities. Job sites in construction zones where heavy machinery is operating are also hazardous, and the dangers are often exacerbated by poor hearing conditions from the excessive noise, and poor visibility caused by obstructions and dust, both of which are created by the construction activity. In these situations, whenever possible, the surveys should be removed from the danger areas through careful planning and/or the use of *offset* lines. If the work must be done in these hazardous areas, then certain safety precautions should be followed. Safety vests of fluorescent yellow color should always be worn in these situations, and flagging materials of the same color can be attached to the surveying equipment to make it more visible. Depending on the circumstances, signs can be placed in advance of work areas to warn drivers of the presence of a survey party ahead, cones and/or barricades can be placed to deflect traffic around surveying activities, and flaggers can be assigned to warn drivers, or to slow or even stop them, if necessary. The *Occupational Safety and Health Administration* (OSHA), of the U.S. Department of Labor,⁴ has developed safety standards and guidelines that apply to the various conditions and situations that can be encountered.

Besides the hazards described above, depending on the location of the survey and the time of year, other dangers can also be encountered in conducting field surveys. These include problems related to weather such as frostbite and overexposure to the sun's rays, which can cause skin cancers, sunburns, and heat stroke. To help prevent these problems, plenty of fluids should be drunk, large-brimmed hats and sunscreen can be worn, and on extremely hot days surveying should commence at dawn and terminate at midday or early afternoon. Outside work should not be done on extremely cold days, but if it is necessary, warm clothing should be worn and skin areas should not be exposed. Other hazards that can be encountered during field surveys include wild animals, poisonous snakes, bees, spiders, wood ticks, deer ticks (which can carry Lyme disease), poison ivy, and poison oak. Surveyors should be knowledgeable about the types of hazards that can be expected in any local area, and always be alert and on the lookout for them. To help prevent injury from these sources, protective boots and clothing should be worn and insect sprays used. Certain tools can also be dangerous, such as chainsaws, axes, and machetes that are sometimes necessary for clearing lines of sight. These must always be handled with care. In addition, care must be exercised in

⁴The mission of OSHA is to save lives, prevent injuries, and protect the health of America's workers. Its staff establishes protective standards, enforces those standards, and reaches out to employers and employees through technical assistance and consultation programs. For more information about OSHA and its safety standards, consult its website <http://www.osha.gov>.

handling certain surveying instruments, like long-range poles and level rods, especially when working around overhead wires, to prevent accidental electrocutions.

Many other hazards, in addition to those cited above, can be encountered when surveying in the field. Thus, it is essential that surveyors always exercise caution in their work, and know and follow accepted safety standards. In addition, a first-aid kit should always accompany a survey party in the field, and it should include all of the necessary antiseptics, ointments, bandage materials, and other equipment needed to render first aid for minor accidents. The survey party should also be equipped with cell phones for more serious situations, and telephone numbers to call in emergencies should be written down and readily accessible.

■ 1.8 LAND AND GEOGRAPHIC INFORMATION SYSTEMS

Land Information Systems (LISs) and GISs are areas of activity that have rapidly assumed positions of major prominence in surveying. These computer-based systems enable storing, integrating, manipulating, analyzing, and displaying virtually any type of spatially related information about our environment. LISs and GISs are being used at all levels of government, and by businesses, private industry, and public utilities to assist in the management and decision-making process. Specific applications have occurred in many diverse areas and include natural resource management, facilities siting and management, land records modernization, demographic and market analysis, emergency response and fleet operations, infrastructure management, and regional, national, and global environmental monitoring. Data stored within LISs and GISs may be both natural and cultural, and be derived from new surveys, or from existing sources such as maps, charts, aerial and satellite photos, tabulated data and statistics, and other documents. However, in most situations, the needed information either does not exist, or it is unsatisfactory because of age, scale, or other reasons. Thus, new measurements, maps, photos, or other data must be obtained.

Specific types of information (also called *themes* or *layers* of information) needed for land and GISs may include political boundaries, individual property ownership, population distribution, locations of natural resources, transportation networks, utilities, zoning, hydrography, soil types, land use, vegetation types, wetlands, and many, many more. An essential ingredient of all information entered into LIS and GIS databases is that it is spatially related, that is, located in a common geographic reference framework. Only then are the different layers of information physically relatable so they can be analyzed using computers to support decision making. This geographic positional requirement will place a heavy demand upon surveyors (geomatics engineers) in the future, who will play key roles in designing, implementing, and managing these systems. Surveyors from virtually all of the specialized areas described in Section 1.6 will be involved in developing the needed databases. Their work will include establishing the required basic control framework; conducting boundary surveys and preparing legal descriptions of property ownership; performing topographic and hydrographic surveys by ground, aerial, and satellite methods; compiling and digitizing maps; and assembling a variety of other digital data files.

The last chapter of this book, Chapter 28, is devoted to the topic of land and geographic information systems. This subject seems appropriately covered at the end, after each of the other types of surveys needed to support these systems has been discussed.

■ 1.9 FEDERAL SURVEYING AND MAPPING AGENCIES

Several agencies of the U.S. government perform extensive surveying and mapping. Three of the major ones are:

1. The National Geodetic Survey (NGS), formerly the Coast and Geodetic Survey, was originally organized to map the coast. Its activities have included control surveys to establish a network of reference monuments throughout the United States that serve as points for originating local surveys, preparation of nautical and aeronautical charts, photogrammetric surveys, tide and current studies, collection of magnetic data, gravimetric surveys, and worldwide control survey operations. The NGS plays a major role in coordinating and assisting in activities related to upgrading the national network of reference control monuments, and to the development, storage, and dissemination of data used in modern LISs and GISs.
2. The U.S. Geological Survey (USGS), established in 1879, has as its mission the mapping of our nation and the survey of its resources. It provides a wide variety of maps, from topographic maps showing the geographic relief and natural and cultural features, to thematic maps that display the geology and water resources of the United States, to special maps of the moon and planets. The National Mapping Division of the USGS has the responsibility of producing topographic maps. It currently has nearly 70,000 different topographic maps available, and it distributes approximately 10 million copies annually. In recent years, the USGS has been engaged in a comprehensive program to develop a national digital cartographic database, which consists of map data in computer-readable formats.
3. The Bureau of Land Management (BLM), originally established in 1812 as the General Land Office, is responsible for managing the public lands. These lands, which total approximately 264 million acres and comprise about one eighth of the land in the United States, exist mostly in the western states and Alaska. The BLM is responsible for surveying the land and managing its natural resources, which include minerals, timber, fish and wildlife, historical sites, and other natural heritage areas. Surveys of most public lands in the conterminous United States have been completed, but much work remains in Alaska.

In addition to these three federal agencies, units of the U.S. Army Corps of Engineers have made extensive surveys for emergency and military purposes. Some of these surveys provide data for engineering projects, such as those connected with flood control. Surveys of wide extent have also been conducted for special purposes by nearly 40 other federal agencies, including the Forest Service, National Park Service, International Boundary Commission, Bureau of

Reclamation, Tennessee Valley Authority, Mississippi River Commission, U.S. Lake Survey, and Department of Transportation.

All states have a surveying and mapping section for purposes of generating topographic information upon which highways are planned and designed. Likewise, many counties and cities also have surveying programs, as have various utilities.

■ 1.10 THE SURVEYING PROFESSION

The personal qualifications of surveyors are as important as their technical ability in dealing with the public. They must be patient and tactful with clients and their sometimes-hostile neighbors. Few people are aware of the painstaking research of old records required before fieldwork is started. Diligent, time-consuming effort may be needed to locate corners on nearby tracts for checking purposes as well as to find corners for the property in question.

Land or boundary surveying is classified as a learned profession because the modern practitioner needs a wide background of technical training and experience, and must exercise a considerable amount of independent judgment. Registered (licensed) professional surveyors must have a thorough knowledge of mathematics (particularly geometry, trigonometry, and calculus); competence with computers; a solid understanding of surveying theory, instruments, and methods in the areas of geodesy, photogrammetry, remote sensing, and cartography; some competence in economics (including office management), geography, geology, astronomy, and dendrology; and a familiarity with laws pertaining to land and boundaries. They should be knowledgeable in both field operations and office computations. Above all, they are governed by a professional code of ethics and are expected to charge professional-level fees for their work.

Permission to trespass on private property or to cut obstructing tree branches and shrubbery must be obtained through a proper approach. Such privileges are not conveyed by a surveying license or by employment in a state highway department or other agency (but a court order can be secured if a landowner objects to necessary surveys).

All 50 states, Guam, and Puerto Rico have registration laws for professional surveyors and engineers (as do the provinces of Canada). In general, a surveyor's license is required to make property surveys, but not for construction, topographic, or route work, unless boundary corners are set.

To qualify for registration as either a professional land surveyor (PLS) or a professional engineer (PE), it is necessary to have an appropriate college degree, although some states allow relevant experience in lieu of formal education. In addition, candidates must acquire two or more years of mentored practical experience and must also pass comprehensive written examinations. In most states, common national examinations covering fundamentals and principles and practice of surveying are now used. However, usually two hours of the principles and practice exam are devoted to local legal customs and aspects. As a result, transfer of registration from one state to another has become easier.

Many states also require continuing education units (CEUs) for registration renewal. Typical state laws require that a licensed land surveyor sign all plats, assume responsibility for any liability claims, and take an *active part* in the fieldwork.

■ 1.11 PROFESSIONAL SURVEYING ORGANIZATIONS

There are many professional organizations in the United States and worldwide that serve the interests of surveying and mapping. Generally, the objectives of these organizations are the advancement of knowledge in the field, encouragement of communication among surveyors, and upgrading of standards and ethics in surveying practice. The National Society of Professional Surveyors (NSPS) represents boundary and construction surveyors in the United States. The *American Association for Geodetic Surveying* (AAGS) represents geodetic surveying professionals and the *Geographic and Land Information Society* (GLIS) represent those involved in GISs. AAGS and GLIS publish the biannual journal *Surveying and Land Information Science* (SaLIS).

As noted in the preceding section, all states require persons who perform boundary surveys to be licensed. Most states also have professional surveyor societies or organizations with full membership open only to licensed surveyors. These state societies are generally affiliated with NSPS and concentrate on matters of state and local concern.

The *American Society for Photogrammetry and Remote Sensing* (ASPRS) is an organization also devoted to the advancement of the fields of measurement and mapping, although its major interests are directed toward the use of aerial and satellite imagery for achieving these goals. Its monthly journal *Photogrammetric Engineering and Remote Sensing* regularly features surveying and mapping articles.

The *Geomatics Division* of the *American Society of Civil Engineers* (ASCE) is also dedicated to professional matters related to surveying and publishes quarterly the *Journal of Surveying Engineering*.

The *Surveying and Geomatics Educators Society* (SAGES) holds pedagogical conferences on the instruction of surveying/geomatics in higher educational institutions. These conferences occur biennially at host institutions throughout the North American continent.

Another organization in the United States, the *Urban and Regional Information Systems Association* (URISA), also supports the profession of surveying and mapping. This organization uses information technology to solve problems in planning, public works, the environment, emergency services, and utilities. Its *URISA Journal* is published quarterly.

The *Canadian Institute of Geomatics* (CIG), formerly the *Canadian Institute of Surveying and Mapping* (CISM), is the foremost professional organization in Canada concerned with surveying. This organization disseminates information to its members through its *Geomatica*.

The *International Federation of Surveyors* (FIG), founded in 1878, fosters the exchange of ideas and information among surveyors worldwide. The acronym

FIG stems from its French name, *Fédération Internationale des Géomètres*. *FIG* membership consists of professional surveying organizations from many countries throughout the world. *FIG* is organized into nine technical commissions, each concerned with a specialized area of surveying. The organization sponsors international conferences, usually at four-year intervals, and its commissions also hold periodic symposia where delegates gather for the presentation of papers on subjects of international interest.

■ 1.12 SURVEYING ON THE INTERNET

The explosion of available information on the Internet has had a significant impact on the field of surveying (geomatics). The Internet enables the instantaneous electronic transfer of documents to any location where the necessary computer equipment is available. It brings resources directly into the office or home, where previously it was necessary to travel to obtain the information or wait for its transfer by mail. Some equipment manufacturers provide cloud access in their instruments to support the transfer of information from and to the field crews. Software, educational materials, technical documents, standards, and much more useful information are available on the Internet. As an example of how surveyors can take advantage of the Internet, data from a *Continuously Operating Reference Station* (CORS) can be downloaded from the NGS website for use in GNSS surveys (see Section 14.3.5).

Many agencies and institutions maintain websites that provide data free of charge on the Internet. Additionally, some educational institutions now place credit and noncredit courses on the Internet so that distance education can be more easily achieved. With a web browser, it is possible to research almost any topic from a convenient location, and names, addresses, and phone numbers of goods or services providers in a specific area can be identified. As an example, if it was desired to find companies offering mapping services in a certain region, a web search engine could be used to locate web pages that mention this service. Such a search may result in over a million pages if a very general term such as “mapping services” is used to search, but using more specific terms can narrow the search.

Unfortunately, the addresses of particular pages and entire sites, given by their *Uniform Resource Locators* (URLs), tend to change with time. However, at the risk of publishing URLs that may no longer be correct, a short list of important websites related to surveying is presented in Table 1.1.

■ 1.13 FUTURE CHALLENGES IN SURVEYING

Surveying is currently in the midst of a revolution in the way data are measured, recorded, processed, stored, retrieved, and shared. This is in large part because of developments in computers and computer-related technologies. Concurrent with technological advancements, society continues to demand more data, with increasingly higher standards of accuracy, than ever before. Consequently, in a few years the demands on surveying engineers (geomatics engineers) will likely be very different from what they are now.

In the future, the National Spatial Reference System, a network of horizontal and vertical control points, must be maintained and supplemented to meet

TABLE 1.1 UNIFORM RESOURCE LOCATOR ADDRESSES FOR SOME SURVEYING RELATED SITES

Uniform Resource Locator	Owner of Site
http://www.ngs.noaa.gov	National Geodetic Survey
http://www.usgs.gov	U.S. Geological Survey
http://www.blm.gov	Bureau of Land Management
http://www.navcen.uscg.mil	U.S. Coast Guard Navigation Center
http://www.usno.navy.mil	U.S. Naval Observatory
http://www.nspss.us.com/	National Society of Professional Surveyors
http://www.aagsmo.org	American Association for Geodetic Surveying
https://www.cig-acsg.ca/	Canadian Institute of Geomatics
http://www.asprs.org	American Society for Photogrammetry and Remote Sensing
http://www.asce.org	American Society of Civil Engineers
http://www.geosages.org/	Surveying and Geomatics Educators Society
http://pipelinesurveyor.org/	Association of Pipeline Surveyors
https://www.asce.org/utility-engineering-and-surveying/utility-engineering-and-surveying-institute/	Utility Engineering and Surveying Institute
https://aagsmo.org/salis-journal/	Surveying and Land Information Science

requirements of increasingly higher-order surveys. New topographic maps with larger scales as well as digital map products are necessary for better planning. Existing maps of our rapidly expanding urban areas need revision and updating to reflect changes, and more and better map products are needed of the older parts of our cities to support urban renewal programs and infrastructure maintenance and modernization. Large quantities of data will be needed to plan and design new rapid-transit systems to connect our major cities, and surveyors will face new challenges in meeting the precise standards required in staking alignments and grades for these systems.

In the future, assessment of environmental impacts of proposed construction projects will call for more and better maps and other data. GISs and LISs that contain a variety of land-related data such as ownership, location, acreage, soil types, land uses, and natural resources must be designed, developed, and maintained. Cadastral surveys of the yet unsurveyed public lands are essential. Monuments set years ago by the original surveyors have to be recovered and remonumented for preservation of property boundaries. Appropriate surveys with very demanding accuracies will be necessary to position drilling rigs as mineral and oil explorations press further offshore. Other future challenges include making precise deformation surveys for monitoring existing structures such as dams, bridges, and skyscrapers to detect imperceptible movements that could be precursors to catastrophes caused by their failure. Timely measurements and maps of the effects of natural disasters such as earthquakes, floods, and hurricanes will be needed so that effective relief and assistance efforts

can be planned and implemented. In the space program, the desire for maps of neighboring planets will continue. In addition, we must increase our activities in measuring and monitoring natural and human-caused global changes (glacial growth and retreat, volcanic activity, large-scale deforestation, and so on) that can potentially affect our land, water, atmosphere, energy supply, and even our climate.

These and other opportunities offer professionally rewarding indoor or outdoor (or both) careers for numerous people with suitable training in the various branches of surveying.

PROBLEMS

NOTE: Answers for some of these problems, and some in later chapters, can be obtained by consulting the bibliographies, later chapters, websites, or professional surveyors.

- 1.1 List 10 uses for surveying in areas other than land surveying.
- 1.2 Explain the difference between geodetic and plane surveys.
- 1.3 List some application of surveying in geology, forestry, and mining.
- 1.4 Why is it important to make accurate surveys of underground utilities?
- 1.5 Discuss the uses for topographic surveys.
- 1.6 What are hydrographic surveys, and why are they important?
- 1.7 Print a view of your location using Google Earth.[®]
- 1.8 Briefly explain the procedure used by Eratosthenes in determining the Earth's circumference.
- 1.9 Describe the steps a land surveyor would need to do when performing a boundary survey.
- 1.10 What is the name of the state-level professional surveying organization in your state or region?
- 1.11 List the tasks of a surveyor as defined by the International Federation of Surveyors?
- 1.12 What organizations in your state furnish maps and reference data to surveyors and engineers?
- 1.13 List the legal requirements for registration as a land surveyor in your state.
- 1.14 Briefly describe an Earth-Centered, Earth-Fixed coordinate system.
- 1.15 List the professional societies representing the geospatial industry in the
(a) United States. (b) Canada. (c) International.
- 1.16 Explain how aerial photographs and satellite images can be valuable in surveying.
- 1.17 List the various specialized types of surveys.
- 1.18 What types of office work will a surveyor encounter?
- 1.19 Visit one of the surveying websites listed in Table 1.1, and write a brief summary of its contents. Briefly explain the value of the available information to surveyors.
- 1.20 Read one of the articles cited in the bibliography for this chapter, or another of your choosing, that describes an application where satellite surveying methods were used. Write a brief summary of the article.
- 1.21 Same as Problem 1.20, except the article should be on safety as related to surveying.

BIBLIOGRAPHY

- Anderson, J. 2012. "Wow! I'm an LSIT! (Uh, Now What Do I Do?)" *The American Surveyor* 11 (No. 5): 23.
- Aust, M. 2014. "A Safe Habitat." *Point of Beginning* 38 (No. 2): 36.
- Blouin, D. 2014. "An Intro to IoT: Internet of Things and Internet of Where-Things-Are." *XYHT* 1 (No. 3): 33.
- Brown, D. and J. Ouellete. 2014. "Aid after Catastrophe." *XYHT Fall Supplement*: 8.
- Cantu, O. R. 2014. "The Cloud: Changing the Dynamic of Field-to-Office Communication." *The American Surveyor* 11 (No. 9): 34.
- Cheves, M. 2014. "Smithsonian X 3D" *The American Surveyor* 11 (No. 4): 16.
- Dahn, R. E. and R. Lumos. 2006. "National Society of Professional Surveyors." *Surveying and Land Information Science* 66 (No. 2): 111.
- Duffy, L. 2014. "RFID: Going Below the Surface." *Point of Beginning* 39 (No. 6): 14.
- Greenfeld, J. 2006. "The Geographic and Land Information Society and GIS/LIS Activities in the United States." *Surveying and Land Information Science* 66 (No. 2): 119.
- Koon, R. 2014. "Safety Sense: How Bad Can It Get after an Accident." *Point of Beginning* 39 (No. 9): 32.
- Lathrop, W. and D. Martin. 2006. "The American Association for Geodetic Surveying: Its Continuing Role in Shaping the Profession." *Surveying and Land Information Science* 66 (No. 2): 97.
- Luccio, M. 2014. "Captured: From Traditional Photogrammetry to UAS." *XYHT* 1 (No. 2): 42.
- _____. 2014. "From Shore to Floor." *XYHT* 1 (No. 4): 16.
- _____. 2014. "Extraterrestrial Photogrammetry." *XYHT* 1 (No. 1): 20.
- _____. 2014. "Precision Ag: Satellite Signals Steer Farmers Straight." *XYHT* 1 (No. 5): 31.
- Morton, C. 2014. "Digging into Archaeology." *The American Surveyor* 11 (No. 6): 38.
- Naberezny, B. 2014. "National Surveyors Week 2014." *Professional Surveyor* 34 (No. 5): 8.
- Paulk, M. 2016. "One Man Crews." *The American Surveyor* 14 (No. 7): 4.
- Plaza, J. 2014. "UAVs in Latin America: All Over the Map." *XYHT* 1 (No. 5): 24.
- Silver, M. 2014. "The Dinosaur Surveyors." *The American Surveyor* 11 (No. 8): 18.
- Velde, G. 2014. "Out of Africa." *The American Surveyor* 11 (No. 2): 16.
- Wilson, B. 2014. "A Dam with a View." *The American Surveyor* 11 (No. 10): 20.

An aerial photograph of a city, likely New York City, showing a dense urban landscape. A semi-transparent wireframe mesh is overlaid on a portion of the city, highlighting a specific building and its surrounding area. The wireframe is composed of numerous small triangles and lines, creating a 3D effect. The background shows various city buildings, streets, and green spaces.

2

Units, Significant Figures, and Field Notes

PART I • UNITS AND SIGNIFICANT FIGURES

2.1 INTRODUCTION

Five types of observations illustrated in Figure 2.1 form the basis of traditional plane surveying: (1) horizontal angles, (2) horizontal distances, (3) vertical (or zenith) angles, (4) vertical distances, and (5) slope distances. In the figure, OAB and ECD are horizontal planes, and $OACE$ and $ABDC$ are vertical planes. Then as illustrated, horizontal angles, such as angle AOB , and horizontal distances, OA and OB , are measured in horizontal planes; vertical angles, such as AOC , are measured in vertical planes; zenith angles, such as EOC , are also measured in vertical planes; vertical lines, such as AC and BD , are measured vertically (in the direction of gravity); and slope distances, such as OC , are determined along inclined planes. By using combinations of these basic observations, it is possible to compute relative positions between any points. Equipment and procedures for making each of these basic kinds of observations are described in later chapters of this book.

2.2 UNITS OF MEASUREMENT

Magnitudes of measurements (or of values derived from observations) must be given in terms of specific units. In surveying, the most commonly employed units are for *length*, *area*, *volume*, and *angle*. Two different systems are in use for specifying units of observed quantities, the *English* and *metric* systems. Because of its widespread adoption, the metric system is called the *International System of Units*, and abbreviated *SI*.

The basic unit employed for length measurements in the English system is the foot, whereas the meter is used in the metric system. In the past, two different

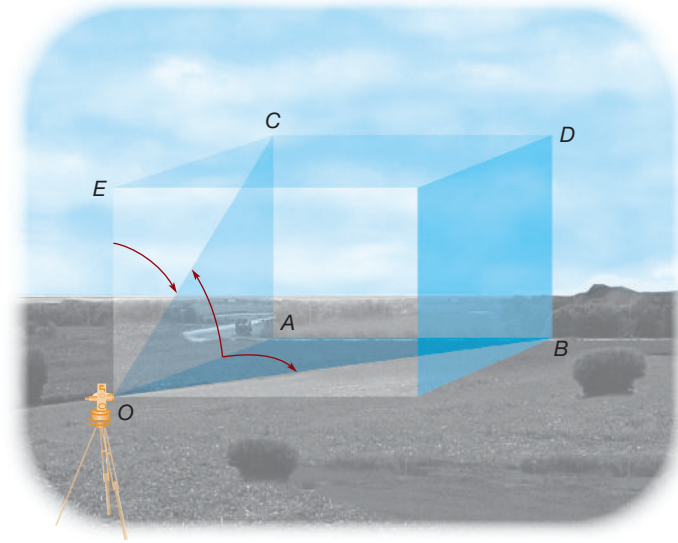


Figure 2.1
Kinds of
measurements in
surveying.

definitions have been used to relate the foot and meter. Although they differ slightly, their distinction must be made clear in surveying. In 1893, the United States officially adopted a standard in which 39.37 in. was exactly equivalent to 1 m. Under this standard, the foot was approximately equal to 0.3048006 m. In 1959, a new standard was officially adopted in which the inch was equal to exactly 2.54 cm. Under this standard, 1 ft equals exactly 0.3048 m. This current unit, known as the international foot, differs from the previous one by about 1 part in 500,000, or approximately 1 foot per 100 miles. This small difference is thus important for very precise surveys conducted over long distances, and for conversions of high elevations or large coordinate values such as those used in State Plane Coordinate Systems as discussed in Chapter 20. Because of the vast number of surveys performed prior to 1959, it would have been extremely difficult and confusing to change all related documents and maps that already existed. Thus the old standard, now called the *U.S. survey foot (sft)*, is still used. Individual states have the option of officially adopting either standard, which is further discussed in Sections 2.3 and 20.2. The National Geodetic Survey uses the meter in its distance measurements; thus, it is unnecessary to specify the foot unit. However, those making conversions from metric units must know the adopted standard for their state and use the appropriate conversion factor.

Because the English system has long been the officially adopted standard for measurements in the United States, except for geodetic surveys, the linear units of feet and *decimals* of a foot are most commonly used by surveyors. In construction, feet and inches are often used. Because surveyors perform all types of surveys including geodetic, and they also provide measurements for developing construction plans and guiding building operations, they must understand all the various systems of units and be capable of making conversions between them. Caution must always be exercised to ensure that observations are recorded in their proper units, and conversions are correctly made.

A summary of the length units used in past and present surveys in the United States includes the following:

1 foot = 12 inches

1 yard = 3 feet

1 inch = 2.54 centimeters (basis of international foot)

1 meter = 39.37 inches (basis of U.S. survey foot)

1 rod = 1 pole = 1 perch = 16.5 feet

1 vara = approximately 33 inches (old Spanish unit often encountered in the southwestern United States)

1 Gunter's chain (ch) = 66 feet = 100 links (lk) = 4 rods

1 mile = 5280 feet = 80 Gunter's chains

1 nautical mile = 1852 m \approx 6076.10 sft \approx 6076.12 ift (nominal length of a minute for any great circle on the Earth)

1 fathom = 6 feet.

In the English system, areas are given in *square feet* or *square yards*. The most common unit for large areas is the *acre*. Ten square chains (Gunter's) equal 1 acre. Thus, an acre contains 43,560 ft², which is the product of 10 and 66². The *arpent* (equal to approximately 0.85 acre, but varying somewhat in different states) was used in land grants of the French crown. When employed as a linear term, it refers to the length of a side of 1 square arpent.

Volumes in the English system can be given in *cubic feet* or *cubic yards*. For very large volumes, for example, the quantity of water in a reservoir, the *acre-foot* unit is used. It is equivalent to the area of an acre having a depth of 1 ft, and thus is 43,560 ft³.

The unit of angle used in surveying is the *degree*, defined as 1/360 of a circle. One degree (1°) equals 60 minutes, and 1 minute equals 60 seconds. Divisions of seconds are given in tenths, hundredths, and thousandths. Other methods are also used to subdivide a circle, for example, 400 *grad* (with 100 *centesimal min/grad* and 100 *centesimal sec/min*). Another term, *gons*, is now used interchangeably with grads. The military services use *mils* to subdivide a circle into 6400 units.

A *radian* is the angle subtended by an arc of a circle having a length equal to the radius of the circle. Therefore, 2π rad = 360°, 1 rad \approx 57°17'44.8" \approx 57.2958°, and 0.01745 rad \approx 1°.

■ 2.3 INTERNATIONAL SYSTEM OF UNITS (SI)

As noted previously, the meter is the basic unit for length in the metric or SI system. Subdivisions of the meter (m) are the *millimeter* (mm), *centimeter* (cm), and *decimeter* (dm), equal to 0.001, 0.01, and 0.1 m, respectively. A kilometer (km) equals 1000 m, which is approximately five eighths of a mile.

Areas in the metric system are specified using the *square meter* (m²). Large areas, for example, tracts of land, are given in *hectares* (ha), where 1 ha is equivalent to a square having sides of 100 m. Thus, there are 10,000 m², or about 2.471 acres/ha. The *cubic meter* (m³) is used for volumes in the SI system. Degrees, minutes, and seconds, or the radian, are accepted SI units for angles.

The metric system was originally developed in the 1790s in France. Although other definitions were suggested at that time, the French Academy of Sciences chose to define the meter as $1/10,000,000$ of the length of the Earth's meridian through Paris from the equator to the pole. The actual length that was adopted for the meter was based on observations that had been made up to that time to determine the Earth's size and shape. Although later measurements revealed that the initially adopted value was approximately 0.2 mm short of its intended definition related to the meridional quadrant, still the originally adopted length became the standard.

Shortly after the metric system was introduced to the world, Thomas Jefferson who was the then secretary of state, recommended that the United States adopt it, but the proposal lost by one vote in the Congress! When the metric system was finally legalized for use (but not officially adopted) in the United States in 1866, a meter was defined as the interval under certain physical conditions between lines on an international prototype bar made of 90% platinum and 10% iridium, and accepted as equal to exactly 39.37 in. A copy of this bar was held in Washington, D.C. and compared periodically with the international standard held in Paris. In 1960, at the General Conference on Weights and Measures (CGPM), the United States and 35 other nations agreed to redefine the meter as the length of 1,650,763.73 waves of the orange-red light produced by burning the element krypton (Kr-86). That definition permitted industries to make more accurate measurements and to check their own instruments without recourse to the standard meter-bar in Washington. The wavelength of this light is a true constant, whereas there is a risk of instability in the metal meter-bar. The CGPM met again in 1983 and established the current definition of the meter as the length of the path traveled by light in a vacuum during a time interval of $1/299,792,458$ seconds. Obviously with this definition, the speed of light in a vacuum becomes exactly 299,792,458 m/sec. The advantage of this latest standard is that the meter is more accurately defined, since it is in terms of time, the most accurate of our basic measurements.

During the 1960s and 1970s, significant efforts were made toward promoting adoption of SI as the legal system for weights and measures in the United States. However, costs and frustrations associated with making the change generated substantial resistance, and the efforts were temporarily stalled. Recognizing the importance to the United States of using the metric system in order to compete in the rapidly developing global economy, in 1988 the Congress enacted the *Omnibus Trade and Competitiveness Act*. It designated the metric system as the *preferred* system of weights and measures for U.S. trade and commerce. The Act, together with a subsequent *Executive Order* issued in 1991, required all federal agencies to develop definite metric conversion plans and to use SI standards in their procurements, grants, and other business-related activities to the extent economically feasible. As an example of one agency's response, the Federal Highway Administration adopted a plan calling for (1) use of metric units in all publications and correspondence after September 30, 1992 and (2) use of metric units on all plans and contracts for federal highways after September 30, 1996. Although the Act and Executive Order did not mandate states, counties, cities, or industries to convert to metric, strong incentives were provided, for example, if SI directives were not complied with, certain federal matching funds could be withheld.

In light of these developments, it appeared that the metric system would soon become the official system for use in the United States. However, again much resistance was encountered, not only from individuals but also from agencies of some state, county, town, and city governments, as well as from certain businesses. As a result, the SI still has not been adopted officially in the United States.

Besides the obvious advantage of being better able to compete in the global economy, another significant advantage that would be realized in adopting the SI standard would be the elimination of the confusion that exists in making conversions between the English System and the SI. The 1999 crash of the Mars Orbiter underscores costs and frustrations associated with this confusion. This \$125 million satellite was supposed to monitor the Martian atmosphere, but instead it crashed into the planet because its contractor used English units while NASA's Jet Propulsion Laboratory was giving it data in the metric system. For these reasons and others, such as the decimal simplicity of the metric system, surveyors who are presently burdened with unit conversions and awkward computations involving yard, foot, and inch units should welcome official adoption of the SI. However, since this adoption has not yet occurred, this book uses both English and SI units in discussion and example problems.

Legislatively, Arizona, Michigan, Montana, North Dakota, Oregon, and South Carolina have adopted the international foot definition for conversions. Alabama and Missouri did not legislatively define the foot. All remaining states adopted the survey foot definition adding confusion to which definition of the foot should be used in conversions. This book will use the survey foot definition unless otherwise specified. It should be noted that sometime after January 1, 2023, the United States will be deprecating the survey foot definition, and thus the surveyor will need to adopt this change according to the epoch of the record data.

■ 2.4 SIGNIFICANT FIGURES

In recording observations, an indication of the accuracy attained is the number of digits (significant figures) recorded. By definition, the number of significant figures in any observed value includes the positive (certain) digits plus one (*only one*) digit that is estimated or rounded off, and therefore questionable. For example, a distance measured with a tape whose smallest graduation is 0.01 ft, and recorded as 73.52 ft, is said to have four significant figures; in this case the first three digits are certain, and the last is rounded off and therefore questionable but still significant.

To be consistent with the theory of errors discussed in Chapter 3, it is essential that data be recorded with the correct number of significant figures. If a significant figure is dropped in recording a value, the time spent in acquiring certain precision has been wasted. On the other hand, if data are recorded with more figures than those that are significant, false precision will be implied. The number of significant figures is often confused with the number of decimal places. Decimal places may have to be used to maintain the correct number of significant figures, but in themselves they do not indicate significant figures. Some examples follow:

Two significant figures: 24, 2.4, 0.24, 0.0024, 0.020

Three significant figures: 364, 36.4, 0.000364, 0.0240

Four significant figures: 7621, 76.21, 0.0007621, 24.00.

Zeros at the end of an integer value may cause difficulty because they may or may not be significant. In a value expressed as 2400, for example, it is not known how many figures are significant; there may be two, three, or four, and therefore definite rules must be followed to eliminate the ambiguity. The preferred method of eliminating this uncertainty is to express the value in terms of powers of 10. The significant figures in the measurement are then written in scientific notation as a number between 1 and 10 with the correct number of zeros and power of 10. As an example, 2400 becomes 2.400×10^3 if both zeros are significant, 2.40×10^3 if one is significant, and 2.4×10^3 if there are only two significant figures. Alternatively, a bar may be placed over the last significant figure, as $\overline{2400}$, $\overline{2400}$, and $\overline{2400}$ for 4, 3, and 2 significant figures, respectively.

When observed values are used in the mathematical processes of addition, subtraction, multiplication, and division, it is imperative that the number of significant figures given in answers be consistent with the number of significant figures in the data used. The following three steps will achieve this for addition or subtraction: (1) identify the column containing the rightmost significant digit in each number being added or subtracted, (2) perform the addition or subtraction, and (3) round the answer so that its rightmost significant digit occurs in the leftmost column identified in step (1). Two examples illustrate the procedure.

(a)	(b)
46.7418	378.
+ 1.03	<u>-2.1</u>
<u>+375.0</u>	375.9
422.7718	(answer 376.)
(answer 422.8)	

In (a), the digits 8, 3, and 0 are the rightmost significant ones in the numbers 46.7418, 1.03, and 375.0, respectively. Of these, the 0 in 375.0 is leftmost with respect to the decimal. Thus, the answer 422.7718 obtained on adding the numbers is rounded to 422.8, with its rightmost significant digit occurring in the same column as the 0 in 375.0. In (b), the digits 8 and 1 are rightmost, and of these the 8 is leftmost. Thus, the answer 375.9 is rounded to 376.

In multiplication, the number of significant figures in the answer is equal to the least number of significant figures in any of the factors. For example, $362.56 \times 2.13 = 772.2528$ when multiplied but the answer is correctly given as 772. Its three significant figures are governed by the three significant digits in 2.13. Likewise, in division the quotient should be rounded off to contain only as many significant figures as the least number of significant figures in either the divisor or the dividend. These rules for significant figures in computations stem from error propagation theory, which is discussed further in Section 3.17.

In surveying, four specific types of problems relating to significant figures are encountered and must be understood.

1. Field measurements are given to some specific number of significant figures, thus dictating the number of significant figures in answers derived when the measurements are used in computations. In an intermediate calculation, it is

a common practice to carry at least one more digit than required, and then round off the final answer to the correct number of significant figures.

2. There may be an implied number of significant figures. For instance, the length of a football field might be specified as 100 yd. But in laying out the field, such a distance would probably be measured to the nearest hundredth of a foot, not the nearest half-yard.
3. Each factor may not cause an equal variation. For example, if a steel tape 100.00 ft long is to be corrected for a change in temperature of 15°F, one of these numbers has five significant figures while the other has only two. However, a 15° variation in temperature changes the tape length by only 0.01 ft. Therefore, an adjusted tape length to five significant figures is warranted for this type of data. Another example is the computation of a slope distance from horizontal and vertical distances, as in Figure 2.2. The vertical distance V is given to two significant figures, and the horizontal distance H is measured to five significant figures. From these data, the slope distance S can be computed to five significant figures. For small angles of slope, a considerable change in the vertical distance produces a relatively small change in the difference between slope and horizontal distances.
4. Observations are recorded in one system of units but may have to be converted to another. A good rule to follow in making these conversions is to retain in the answer a number of significant figures equal to those in the observed value. As an example, to convert 178 ft 6-3/8 in. to m, the number of significant figures in the measured value would first be determined by expressing it in its smallest units. In this case, 1/8th in is the smallest unit and there are $(178 \times 12 \times 8) + (6 \times 8) + 3 = 17,139$ of these units in the value. Thus, the measurement contains five significant figures, and the answer is $17,139 \div (8 \times 39.37 \text{ in./m}) = 54.416 \text{ m}$, properly expressed with five significant figures. (Note that 39.37 used in the conversion is an exact constant and does not limit the number of significant figures.)

■ 2.5 ROUNDING OFF NUMBERS

Rounding off a number is the process of dropping one or more digits so the answer contains only those digits that are significant. In rounding off numbers to any required degree of precision in this text, the following procedures will be observed:

1. When the digit to be dropped is lower than 5, the number is written without the digit. Thus, 78.374 becomes 78.37. Also 78.3749 rounded to four figures becomes 78.37.
2. When the digit to be dropped is exactly 5, the nearest even number is used for the preceding digit. Thus, 78.375 becomes 78.38 and 78.385 is also rounded to 78.38.

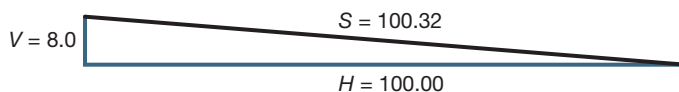


Figure 2.2
Slope correction.

3. When the digit to be dropped is greater than 5, the number is written with the preceding digit increased by 1. Thus, 78.386 becomes 78.39.

Procedures 1 and 3 are standard practice. When rounding the value 78.375 in procedure 2, however, some people always take the next higher hundredth, whereas others invariably use the next lower hundredth. However, using the nearest even digit establishes a uniform procedure and produces better-balanced results in a series of computations. It is an improper procedure to perform two-stage rounding where, for example, in rounding 78.3749 to four digits it would be first rounded to five figures, yielding 78.375, and then rounded again to 78.38. The correct answer in rounding 78.3749 to four figures is 78.37.

It is important to recognize that rounding should only occur with the final answer. Intermediate computations should be done without rounding to avoid problems that can be caused by rounding too early. Example (a) of Section 2.4 is repeated below to illustrate this point. The sum of 46.7418, 1.03, and 375.0 is rounded to 422.8 as shown in the “correct” column. If the individual values are rounded prior to the addition as shown in the “incorrect” column, the incorrect result of 422.7 is obtained.

Correct	Incorrect
$ \begin{array}{r} 46.7418 \\ + 1.03 \\ + 375.0 \\ \hline 422.7718 \\ \text{(answer 422.8)} \end{array} $	$ \begin{array}{r} 46.7 \\ + 1.0 \\ + 375.0 \\ \hline 422.7 \\ \text{(answer 422.7)} \end{array} $

PART II • FIELD NOTES

2.6 FIELD NOTES

Field notes are the records of work done in the field. They typically contain measurements, sketches, descriptions, and many other items of miscellaneous information. In the past, field notes were prepared exclusively by hand lettering in field books or special note pads as the work progressed and data were gathered. However, survey controllers, also known as data collectors and electronic field books, have been introduced that can interface with many different modern surveying instruments. As the work progresses, they create computer files containing a record of observed data. All surveying controllers provide a mapping feature (see Chapter 17). Some controllers and total stations also provide a camera so that an image of the area where data is being collected can be captured. When these features are absent manually prepared sketches and descriptions often supplement the numerical data they capture. Regardless of the manner or form in which the notes are taken, they are extremely important.

Whether prepared manually, created by a survey controller, or a combination of these forms, surveying field notes are the only permanent records of work

done in the field. If the data are incomplete, incorrect, lost, or destroyed, much or all of the time and money invested in making the measurements and records have been wasted. Hence, the job of data recording is frequently the most important and difficult one in a surveying party. Field books and computer files containing information gathered over a period of weeks are worth many thousands of dollars because of the costs of maintaining personnel and equipment in the field.

Recorded field data are used in the office to perform computations, make drawings, or both. The office personnel using the data are usually not the same people who took the notes in the field. Accordingly, it is essential that without verbal explanations notes be intelligible to anyone.

Property surveys are subject to court review under some conditions, so field notes become an important factor in litigation. Also, because they may be used as references in land transactions for generations, it is necessary to index and preserve them properly. The salable “goodwill” of a surveyor’s business depends largely on the office library of field books. Cash receipts may be kept in an unlocked desk drawer, but field books are stored in a fireproof safe!

■ 2.7 GENERAL REQUIREMENTS OF HANDWRITTEN FIELD NOTES

The following points are considered in appraising a set of field notes:

Accuracy. This is the most important quality in all surveying operations.

Integrity. A single omitted measurement or detail can nullify use of the notes for computing or plotting. If the project was far from the office, it is time consuming and expensive to return for a missing measurement. Notes should be checked carefully for completeness before leaving the survey site and never “fudged” to improve closures.

Legibility. Notes can be used only if they are legible. A professional-looking set of notes is likely to be professional in quality.

Arrangement. Note forms appropriate to a particular survey contribute to accuracy, integrity, and legibility.

Clarity. Advance planning and proper field procedures are necessary to ensure clarity of sketches and tabulations, and to minimize the possibility of mistakes and omissions. Avoid crowding notes; paper is relatively cheap. Costly mistakes in computing and drafting are the end results of ambiguous notes.

Appendix B contains examples of handwritten field notes for a variety of surveying operations. Their plate number identifies each. Other example note forms are given at selected locations within the chapters that follow. These notes have been prepared keeping the above points in mind.

In addition to the items stressed in the foregoing, certain other guidelines must be followed to produce acceptable handwritten field notes. The notes should be lettered with a sharp pencil of at least 3H hardness so that an indentation is made in the paper. Books so prepared will withstand damp weather in the field (or even a soaking) and still be legible, whereas graphite from a soft pencil, or ink from a pen or ballpoint, leaves an undecipherable smudge under such circumstances.